## Lindenwold Station Transit Hub Study



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## Executive Summary

## The Study

Lindenwold Station is the junction of two rail lines; the New Jersey Transit Atlantic City line and the Port Authority Transit Corporation (PATCO) High Speed Line, which terminates at Lindenwold. The Lindenwold Station has the highest number of boards for PATCO service in New Jersey. NJ Transit ridership between Philadelphia and Atlantic City has been growing steadily over recent years. Additionally, local bus routes (403, 451, 459, and 554) stop at Lindenwold.

Existing bottlenecks were evaluated, including the CR 673 overpass, and the complex intersections along CR 673 near the station.

A study advisory committee was established to guide this study with representation from both public and private entities in the area surrounding the station.

Recommendations being made by the study team to enhance the existing services available at the station and to support Lindenwold as a hub for transit service are discussed below:
© The study recommends that improvements be made to add capacity at the CR 673 intersections with Berlin Road and US 30. This can be accomplished by restricting southbound turns at the US 30 intersection and converting both southbound lanes to through lanes between Berlin and US 30 .
(4) The operation of the intersection of CR 673 with Station Avenue and the Lindenwold station access drive can be greatly improved by installing a traffic signal that operates during the PM peak. The signal will also allow a safe access point for pedestrians and bicyclists to reach the station from the areas north and west of the station.
() Improvement in the overall delay at the US 30 and New Road intersection can be accomplished by removing the jughandle and providing left-turn lanes along US 30. This can be done without additional pavement width.
() Pedestrian access improvements outlined within the study include the addition of sidewalks along the perimeter of the station and installation of crosswalk marking and countdown signals at signalized intersections. Additionally, improvements are identified for the internal station area including curb ramps, pedestrian pathway delineation, additional sheltered bus stop waiting area, and traffic calming elements.

人) Bicycle facility improvements include the installation of wayfinding signage directing cyclists to the Lindenwold station, as well as to other attractions in the area such as UMDNJ and Kennedy Hospital. "Share The Road" signage is also being recommended by the study team.

## Introduction

## The Lindenwold Station Transit Hub Study

## Goals

The goals of this study are to improve mobility and enhance transit connections to destinations in the developing eastern end of Camden County. The study will determine ways to make access to the station more efficient and take full advantage of connections available between PATCO rail service, New Jersey Transit bus and Atlantic City rail service at the Lindenwold Station.

## Background

Several previous studies have been conducted in the area. A short synopsis of the more recent studies that relate to this study are included here to give some background.

PATCO Parking Study - 2002
This study determined the current and future parking demands at PATCO rail stations and identified potential strategies for increasing parking. Some of the findings for the Lindenwold station included:

- That the overall occupancy for all 3,337 spaces was 90 percent full by 10:30 AM. The paid parking was 100 percent full and the free parking was 88 percent full. The handicap parking area was full by 8:00 AM.
- Congestion on the roadway network surrounding the station contributes to poor levels of service at the station drives. The heavy through movements on CR 673 and Berlin Road reduce the green time for exiting station traffic and limits the availability of acceptable gaps for the unsignalized intersections.
- Backups from the signalized intersections frequently extend past the station drives.
- Traffic appears to use the PATCO drives as a bypass around the congested traffic signals.


## CR 673 Arterial Progression Study - 2005

This study recommended a wireless interconnect system to accomplish arterial progression along the CR 673 corridor south of US 30. Cycle lengths of 120 seconds were recommended during the peak hours to increase speeds and Level of Service for the northbound direction in the AM peak and southbound direction in the PM peak. The CR 673 and Medical Center Drive intersection was included in this study. Traffic signal timing modifications were recommended for the intersection, including different timing for AM and PM peak hours. These timing recommendations were not implemented due to the fact that the study was not advanced to the design phase.

## DVRPC Increasing Intermodal Access to Transit, Phase II - 2005

This study assessed the non-motorized access to transit stations, including the Lindenwold Station. Road and sidewalk facilities surrounding the station were assessed within one mile and one quarter mile distances respectively. Bicycle Level of Service and Pedestrian Level of Service measures were used to qualify the conditions facing bicyclists and pedestrians. The large parking facility, the narrow shoulders, sidewalks without buffers, and high operating speeds all contributed to poor LOS results in the area surrounding the Lindenwold Station.

## PATCO Transit Oriented Development Master Plans Study - 2006

The study examined the feasibility of replacing the current surface parking lots adjacent to the Lindenwold station with transit oriented development comprised of office, retail, and residential uses. The market and financial feasibility of this plan was determined, as well as the effects of such development on the station and the Borough of Lindenwold. Focusing mainly on the current and potential future land use patterns, the study concluded that resultant new activity would have positive effects on the pedestrian experience in the area but would likely increase current circulation issues. These issues would need to be addressed through significant infrastructure improvements, including changes to the street network, Berlin and White Horse Road updates, and new intersection signalizations.

## DVRPC Survey Support for PATCO Transit Extension Study - 2008

This study analyzed the results of license plate surveys at three PATCO stations, including the Lindenwold Station, and compared them to a 2001 study in order to determine whether the geographic distribution of riders had shifted in recent years. The license plate survey results revealed that there was no significant geographic shift of patrons between 2001 and 2006, but that there was a lower level of parking occupancy in 2006.

## Study Area

The area being studied includes the Boroughs of Lindenwold, Somerdale, and Stratford, as well as Voorhees Township. Major employers in the area include the University of Medicine and Dentistry of New Jersey (UMDNJ) and Kennedy Hospital.

The boundaries of the study area are:

- To the East - Washington Avenue in Lindenwold
- To the West - Cornell Avenue in Somerdale
- To the North - A line between the PATCO/NJ Transit rail line and Washington Avenue in Voorhees/Lindenwold
- To the South - The Southeast boundary of Stratford
- The closely spaced intersections of CR 702 with Gibbsboro Road under the Atlantic City rail line are also included in the study.

The study area is illustrated in Figure 1, shown on the next page.


## Transportation Analysis

## Arterial Network

The study area is served by a combination of state, county, and municipal roads that provide mobility and access to vehicular traffic traveling through the area. The principal routes are as follows:

CR 673 (Laurel Road/ White Horse Road)
CR 673 is a north-south urban minor arterial that is named Laurel Road south of US 30 and White Horse Road north of US 30. The cross section changes from two lanes to three lanes at the intersection with US 30. The posted speed limit is 25 mph throughout the study area. This roadway is particularly congested during peak periods.

## CR 702 (Berlin Road/Egg Harbor Road)

CR 702 is an urban minor arterial that runs east-west through Camden County. The cross section is two lanes with auxiliary lanes at major intersections. There is no posted speed limit along the roadway within the study area.

## CR 686 (Gibbsboro Road)

CR 686 is an urban minor arterial that runs north-south linking Clementon Borough with Gibbsboro and areas north. The study area includes approximately 160 feet of roadway between the Berlin Road and the Egg Harbor Road intersections under the Atlantic City rail line overpass. The posted speed limit within this area is 25 mph . This roadway sees congestion in both the AM and PM peak periods.

## US 30 (White Horse Pike)

US 30 carries the name White Horse Pike throughout the study area and is classified as an urban principal arterial. This major east-west roadway is a four to five lane cross section with a posted speed of 40 mph .

## Traffic Volumes

Turning Movement count data was collected by DVRPC during peak periods at the following locations. The numbers of the counts correspond to the location numbers shown on Figure 2.

- 1. CR 673 at CR 702
- 2. CR 673 at US 30
- 3. CR 673 at Station Avenue
- 4. CR 702 at station access drive
- 5. CR 702 at New Road
- 6. New Road at US 30
- 7. CR 673 at Central Avenue
- 8. CR 673 at Medical Center Drive
- 9. CR 686 at CR 702 (Berlin Road)
- 10.CR 686 at CR 702 (Egg Harbor Road)

Volume count data was collected by DVRPC during peak periods at several locations including:

- 2a./2b. CR 673 at US 30 jughandle
- 5a. New Road at US 30 jughandle

In order to fully understand the operational conditions within the study area as a whole, the AM and PM peak period traffic volumes were evaluated as a system of all intersections within the corridor. This was done because each individual intersection within the study area had a peak hour that was different from other nearby intersections. To determine the system-wide peak hour, the traffic counts from individual intersections were compiled by hour and an overall peak hour was computed. This system-wide peak hour, which is 7:45-8:45 AM and 4:30-5:30 PM [8:00 - 9:00 AM and 5:00-6:00 PM for Gibbsboro Road (CR 686)], was used in the analysis for this report.

Raw traffic volume data, peak hour tabulations, and calculations used to determine the systemwide peak hour volumes can be found in Appendix A.

Figure 3, shown on the following pages, shows the system-wide peak hour turning movement volumes for each intersection.


Figure 3: Turning Movement Counts


Figure 3: Turning Movement Counts (Continued)


Figure 3: Turning Movement Counts (Continued)


## LEGEND

## Crash Analysis

DVRPC conducted a crash analysis for the Lindenwold Station area, utilizing data collected by the New Jersey Department of Transportation. An analysis of data over a three-year period (2005-2007) reveals 237 reportable crashes in the study area. Table 1 and Table 2 outline the major crash locations while Figure 4 illustrates study area crash numbers along all links and main intersections within the study area. The major crash locations are the top five intersections and top five link locations within the study area. Highway crashes within the study area are concentrated primarily around major intersections, although not necessarily at the intersection, which is defined as between the stop bars. Also, the study area generally has a larger percentage of injury crashes compared to the 2007 crash summary for the entire county road system of New Jersey although no fatal crashes occurred over this period.

## Intersections

The definition of an intersection for the analysis is the area between the stop bars as well as the intersection approaches.

Intersection of CR 673 (White Horse Road) and US 30 (White Horse Pike)
This intersection is the site of the most crashes in the study area. Over the three year period, 48 crashes occurred. Forty-two percent of crashes were rear-end crashes, which is considerably above the statewide county road level of only about 30 percent of crashes being rear-ends. Left-turn/U-turn crash levels are also high for this intersection at 21 percent compared to about 6 percent statewide. Finally, 25 crashes, or 52 percent, resulted in injuries at this intersection. For county roads statewide, only 28 percent of crashes result in injury.

Intersection of CR 673 (White Horse Road) and CR 702 (Berlin Road)
This intersection had the second most crashes in the study area over the three year period. Crashes here were distributed between rear-ends, left-turns/U-turns and right-angle crashes. Having 30 percent of left-turn/U-turn crashes places it well above the statewide county road percentage of about 6 percent and is likely a result of drivers making risky movements after experiencing long delays due to congestion.

Intersection of US 30 and CR 702 (Berlin Road)
At this intersection 27 crashes occurred over the three year period, making it the third largest crash cluster in the study area. Likely as a result of the geometry of the intersection and the fact that it is unsignalized, it has 30 percent of right-angle crashes. Thirty-three percent of crashes are same direction rear-end, the highest percentage for this area.

Intersection of Gibbsboro Road and Berlin Road/Egg Harbor Road
Finally, the Gibbsboro intersections had 26 crashes between 2005 and 2007. The largest numbers of crashes are right-angle crashes with nine, or 35 percent of the total crashes for the intersection. This is compared to about 20 percent statewide. Also, 19 percent of crashes were left-turn/U-turn; again well above the statewide percentage.

Intersection of US 30 and New Road
This intersection had 24 crashes total over the three year period. Compared to statewide county road crash data, it has few right-angle crashes, with only eight percent compared to about 20 percent statewide. Seventy-one percent of crashes occurred not at the intersection, meaning not between the stop bars. Finally, two crashes involving pedestrians occurred, accounting for eight percent of crashes at this intersection.

## Links

The links analyzed do not include any intersections that fall within the boundaries of the link.

US 30 (MP 11.74-11.93) - Between the intersection areas of CR 673 and New Road
This was the site of 13 crashes over the three year period, making it the link with the highest number of crashes in the study area. Forty-six percent of these crashes were rear-end crashes. Also, one crash involving a pedestrian occurred on this segment. For all crashes, 62 percent involved injuries, well above the percentage for county roads statewide.

CR 673 (MP 4.43-4.57) - Between the intersection area of Medical Center Drive and Kirkwood Avenue

This link had the second highest number of crashes with ten total. Nine of the ten were rear-end crashes. All ten crashes occurred in the day time.

CR 673 (MP 4.58-4.66) - Between the intersection areas of Kirkwood Avenue and US 30

This link had five total crashes over the three years, 40 percent each from rear-end and sideswipe crashes.

CR 673 (MP 4.25-4.35) - Between the intersection of Chestnut Avenue and the intersection area of Medical Center Drive

All of the crashes on this link were attributed to the intersection of Chestnut and CR 673. There were four crashes there over the three year period.

CR 702 (MP . 16 - . 34) - Between the intersection areas of CR 673 and New Road
This link abuts the Lindenwold Station and its parking lots. Over the three year period, there were 4 crashes along this link. All four crashes were injury crashes, including one involving a pedestrian. Half of the crashes occurred during the day and the other half occurred at night.

Table 1: Intersection Crash Analysis

|  | Intersections |  |  |  |  | Total for All Intersections | Percent of Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Crash Characteristics | $\begin{gathered} 673 / \\ 30 \end{gathered}$ | 672/702 | 30/Berlin (702) | $\begin{aligned} & \text { Gibbsboro } \\ & \text { I702 } \end{aligned}$ | 30/New Rd |  |  |
| Rear-End (Same Direction) | 20 | 12 | 9 | 7 | 12 | 69 | 35 \% |
| Sideswipe (Same Direction) | 5 | 3 | 4 | 0 | 4 | 17 | $9 \%$ |
| Right-Angle | 8 | 10 | 8 | 9 | 2 | 51 | 26 \% |
| Opposite Direction | 1 | 0 | 0 | 1 |  | 3 | 2 \% |
| Left-Turn/U-Turn | 10 | 11 | 3 | 5 | 3 | 40 | 20 \% |
| Pedestrian | 2 | 0 | 0 | 0 | 2 | 4 | 2\% |
| Other | 2 | 1 | 3 | 4 | 1 | 13 | 7 \% |
| At Intersection | 16 | 16 | 12 | 13 | 7 | 90 | 46 \% |
| Not at Intersection | 32 | 21 | 15 | 13 | 17 | 107 | 54 \% |
| Day | 29 | 32 | 20 | 17 | 18 | 141 | 72 \% |
| Dusk | 0 | 0 | 1 | 0 | 0 | 1 | 1 \% |
| Night | 19 | 5 | 6 | 9 | 6 | 52 | 26 \% |
| Dawn | 0 | 0 | 0 | 0 | 0 | 3 | 2 \% |
| Fatality | 0 | 0 | 0 | 0 | 0 | 0 | 0 \% |
| Injury | 25 | 14 | 9 | 11 | 11 | 87 | 44 \% |
| Property | 23 | 23 | 18 | 15 | 13 | 110 | 56 \% |
| Dry | 34 | 25 | 21 | 22 | 18 | 149 | 76 \% |
| Wet | 14 | 11 | 5 | 4 | 6 | 46 | 23 \% |
| Other | 0 | 1 | 1 | 0 | 0 | 2 | 1 \% |
| Total: | 48 | 37 | 27 | 26 | 24 | 197 | 100 \% |

Source: DVRPC 2009

Table 2: Link Crash Analysis

|  | Links |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Crash Characteristics | 30 <br> between 673 and New | 673 <br> between Medical Ctr. and Kirkwood | 673 <br> between Kirkwood and 30 | 673 <br> between Chestnut and Medical Ctr. | 702 between 673 and New | Total for All Links | Percent of Total |
| Rear-end (Same Direction) | 6 | 9 | 1 | 2 | 0 | 20 | 50 \% |
| Sideswipe (Same Direction) | 3 | 0 | 2 | 1 | 0 | 6 | 15 \% |
| Right-Angle | 1 | 0 | 2 | 1 | 1 | 7 | 18 \% |
| Opposite Direction | 0 | 0 | 0 | 0 | 0 | 0 | 0 \% |
| Left-Turn/U-Turn | 2 | 1 | 0 | 0 | 1 | 4 | 10 \% |
| Pedestrian | 1 | 0 | 0 | 0 | 1 | 2 | 5\% |
| Other | 0 | 0 | 5 | 0 | 1 | 1 | 0\% |
| At Intersection | 2 | 2 | 1 | 4 | 2 | 12 | 30 \% |
| Not at Intersection | 11 | 8 | 4 | 0 | 2 | 28 | 70 \% |
| Day | 7 | 10 | 5 | 2 | 2 | 29 | 73 \% |
| Dusk | 1 | 0 | 0 | 1 | 0 | 2 | 5 \% |
| Night | 5 | 0 | 0 | 1 | 2 | 9 | 23 \% |
| Fatality | 0 | 0 | 0 | 0 | 0 | 0 | 0 \% |
| Injury | 8 | 5 | 2 | 2 | 0 | 24 | 60 \% |
| Property | 5 | 5 | 3 | 2 | 4 | 16 | 40 \% |
| Dry | 12 | 9 | 4 | 4 | 4 | 37 | 93 \% |
| Wet | 1 | 1 | 1 | 0 | 0 | 3 | 8 \% |
| Other | 0 | 0 | 0 | 0 | 0 | 0 | 0 \% |
| Total: | 13 | 10 | 5 | 4 | 4 | 40 | 100 \% |

Source: DVRPC 2009


## Level of Service

The existing conditions of the study area were analyzed by DVRPC in order to evaluate the current traffic operations. This evaluation was conducted using the Level of Service (LOS) procedure. LOS analysis is a qualitative measure of operational conditions within a traffic stream. There are six defined levels of service, A - F, which describe operations from best to worst for the facility under analysis. These levels are defined in terms of parameters perceived by drivers and a range of operating conditions.

## Intersection LOS

LOS of intersections is based on the control delay per vehicle imposed by the intersection. Table 3 shows the criteria for the LOS at signalized and unsignalized intersections. Although the criteria measured for both types of intersections is the control delay per vehicle, the value of the criteria vary due to the fact that drivers perceive delay differently at signalized intersections than unsignalized or stop controlled intersections. A driver expects a different level of performance for a signalized intersection, the expectation is that signalized intersections carry a higher volume of vehicles and therefore a higher value of delay is considered acceptable.

Table 3: Level of Service Criteria for Intersections

| Signalized Intersection | Control Delay <br> LOS <br> Seconds / vehicle | Unsignalized <br> Intersection <br> LOS | Control Delay |
| :---: | :---: | :---: | :---: |
| A | $\leq 10$ | A | Seconds / vehicle |
| B | $>10-20$ | B | $0-10$ |
| C | $>20-35$ | C | $>10-15$ |
| D | $>35-55$ | D | $>15-25$ |
| E | $>55-80$ | E | $>25-35$ |
| F | $>80$ |  | $>35-50$ |
| Source: Highway Capacity Manual 2000 | $>50$ |  |  |

In order to fully understand the operational conditions within the study area, the AM and PM peak period LOS was evaluated as a system of all intersections within the area. As discussed previously, peak hour turning movement counts were compiled in order to determine the peak hour of the system as a whole. This system wide peak hour was used in the analysis (the Gibbsboro Road intersections were analyzed as a separate system than the other intersections due to their distance from the station). This system peak hour volume data, as well as traffic signal information was analyzed using Synchro Software to determine the LOS.


Backup along southbound CR 673 approaching US 30 during PM peak hour

Ten intersections in the study area were analyzed. The overall LOS under existing conditions was determined for the seven signalized intersections within the study area. The results of this analysis are shown in Table 4.

Table 4: Overall Existing Level of Service

| Signalized Intersection | AM System Peak |  | PM System Peak |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Delay (sec.) | LOS | Delay (sec.) | LOS |
| CR 673 \& CR 702 | 37 | D | 217 | F |
| CR 673 \& US 30 | 22 | C | 26 | C |
| CR 702 \& New Road | 21 | C | 22 | C |
| New Road \& US 30 | 10 | A | 45 | D |
| CR 673 \& Medical Center Drive | 374 | F | 412 | F |
| CR 686 \& CR 702 | 32 | C | 22 | C |
| CR 686 \& CR 702 | 28 | C | 30 | C |

Source: DVRPC 2009

Additionally, analysis was broken down by approach of each intersection in order to determine which approaches were suffering the most delay. Five of the intersections were found to be experiencing delays of 80 seconds or more (LOS F) on one or more approaches. These intersections include:

- CR 673 at Berlin Road - Southbound through/right approach toward US 30 in the AM and PM peak hours; westbound approaches in the PM peak hour;
- CR 673 at Station Avenue - southbound approaches toward Berlin Road during AM and PM peak hours, as well as the westbound left-turn and eastbound side-street approaches in both the AM and PM peak hours;
- US 30 at New Road - Southbound left-turn toward Stratford Avenue and right-turn toward Laurel Road in the PM peak hour;
- CR 673 at Medical Center Drive - Northbound approaches toward Berlin Road during both the AM and PM peak hours; and
- CR 686 at CR 702 - Northbound approaches toward Gibbsboro in the AM peak and Southbound approaches toward US 30 in the PM peak hour.


## Analysis and Recommended Improvements

The LOS for existing conditions was compared against potential alternative improvements to determine the best recommendation for each intersection. The Synchro Software's micro simulation model SimTraffic was used in the analysis of recommendations. Traffic signal timing and geometric information was input into the Synchro software and SimTraffic simulations were run using both existing and possible alternative scenarios. The average delay over several simulation runs was used for the LOS determination of each scenario. These results were compared in order to determine which alternative recommendation would have the greatest reduction in delay for the intersection. Appendix B contains LOS tables listing the results of this analysis for each intersection.

Location \#1: CR 673 (White Horse Road) at CR 702 (Berlin Road)
Location \#2: CR 673 (White Horse Road) at US 30 (White Horse Pike)
These two intersections are closely spaced and operate on the US 30 coordinated signal timing plan. Therefore, the east/west movements receive a large proportion of the green time at each intersection. Because the traffic signals at these intersections are coordinated, the controller at the Berlin Road intersection shows a yellow indication to the east/west movements of Berlin Road at the same time as the controller at US 30 shows a yellow indication to the east/west movements of US 30 .

Observations made during field visits showed that vehicles making left-turns from westbound Berlin Road to southbound CR 673 fill the storage area between US 30 and Berlin Road during most cycles. This prevents through vehicles from passing through the intersection until the signal at US 30 turns green and the queue clears. The resulting backup of southbound vehicles was observed to extend past the bridge over the railroad by 4:15 PM. Rear-end accidents occurring at this location are likely due to the fact that the southbound through/right lane backs up to the railroad overpass even during mid-day traffic conditions.

The study team witnessed vehicles traveling southbound on CR 673 turning left onto Berlin Road in front of northbound vehicles during the permitted movement. This indicates that the drivers are not expecting the eventual lagging protected movement. The protected left-turn movement indication (green left-turn arrow) is shown to drivers after the completion of the north/south traffic indication and the northbound only indication. Additionally, motorists turning left onto Berlin Road from southbound CR 673 must maneuver a grade change, thus must make this turn at a slower speed.

The Route 451 bus turns right from westbound Berlin Road to northbound CR 673 in front of the Pufferbelly Restaurant. This is a very tight turn with only ten-foot lanes, and the bus has difficulty making the turn when there is a vehicle in the southbound left-turn lane.

Potential alternatives considered for this intersection included the addition of a southbound rightturn lane at Berlin Road and restriction of southbound turns at the US 30 intersection. All of the alternative recommendations assumed that the preferred improvements discussed for the Station Avenue intersection (location \#3) were implemented. Restricting all southbound turns at the US 30 intersection allows both southbound lanes to be used by through traffic. Based on the LOS analysis of the potential scenarios, the study team is recommending that the southbound rightturn lane be constructed and southbound turns be restricted at US 30. Motorists are currently using the Berlin Road jughandle to westbound US 30, and can access eastbound US 30 by turning left at the Berlin Road intersection then right at New Road and finally left onto eastbound

US 30 from southbound New Road. The addition of the left-turning traffic headed to eastbound US 30 does not adversely impact the operation of either of the New Road traffic signals.

In order to accomodate two southbound through lanes at the US 30 intersection, an additional southbound lane will need to be constructed along Laurel Road. A length of 300 feet is adequate to allow additional capacity at the signal and will only reduce the width of the existing median between Laurel Road and Union Avenue. These improvements will increse the capacity and reduce the overall delay at the CR 673 intersections.

The recommended improvements for these intersections are illustrated on Figure 5.
人) Immediate Recommendations

- Restripe crosswalk markings at the intersections to make crossings more visible.
- Install curb ramps and pedestrian signals with countdown timers.
- Stripe the intersection with 'Do Not Block the Box' markings to prevent motorists from blocking the intersection.

Long-Term Recommendations

- Add a southbound right-turn lane at Berlin Road.
- Install signage at the Berlin Road intersection to direct southbound CR 673 traffic to US 30. Also install signage at New Road to direct eastbound traffic to US 30.
- Install "No Turns" signage and modify curb radii to visually prohibit turns at the US 30 intersection.
- Add capacity along southbound Laurel Road by adding another southbound lane that is approximately 300 feet long.
- The Pufferbelly Restaurant site at the northeast corner of the intersection has preliminary approval for redevelopment as a Walgreen's Pharmacy. The developer is being required to provide sidewalks along the frontage. The redevelopment of this site allows for the potential modification of the intersection radius to accommodate buses turning right onto northbound CR 673.


## Figure 5

CR 673 Recommendations - US 30 and CR 702


## Location \#3: CR 673 at Station Avenue

This unsignalized intersection is the only access drive into the Lindenwold station from CR 673. The CR 673 northbound curb lane ends just before the station entrance. Only station signing and a utility pole would need to be moved to extend this lane to the station entrance. PATCO and Camden County have discussed reconfiguring this lane drop, and PATCO is planning to extend the curb lane to the station drive as part of an upcoming pavement rehabilitation project.

The bridge over the rail lines to the north of the intersection has sidewalk only on the station side of the street; signage prohibits pedestrians on the opposite side of the roadway.


Restricted sight distance at station access drive along CR 673

The southbound left-turn movement into the station is a heavy movement, accounting for more than 25 percent of the approach traffic. This may be because of cut-through traffic attempting to avoid the signals along CR 673 at Berlin Road and US 30. Delaware River Port Authority (DRPA) Police have begun ticketing drivers that use the station area as a cut-through. The left-turn onto CR 673 from the station access drive is difficult to perform in the PM peak hour due to the backup in the southbound direction.

A traffic signal warrant analysis was conducted at this location based on current traffic volumes. Warrant \#3, the Peak Hour Warrant, is the only signal warrant currently met at this location. The traffic volumes between the hours of 5:00 and 7:00 PM are such that a traffic signal is warranted only during these peak hours. The installation of a traffic signal at this location will require signage to warn drivers headed southbound on CR 673 of the traffic signal because of the sight distance restriction caused by the railroad overpass. It is recommended that "Signal Ahead" signs or electronic "Red Signal Ahead" signs be installed as a warning to drivers of possible stopped traffic ahead. Data used for the signal warrant analysis is located in Appendix A.

The recommended improvements for this intersection are illustrated on Figure 6.

- Immediate Recommendations
- DRPA Police should continue ticketing drivers that cut-through the station. It is also recommended that raised intersections and/or crosswalks be installed along the station's internal roadways to further deter cut-through traffic.
- Stripe the intersection with 'Do Not Block the Box' markings to prevent motorists from blocking the intersection.
- Restripe crosswalk markings at the intersection to make crossings more visible.


## () Long-Term Recommendations

- Extend the northbound curb lane to the station access drive.
- Install a traffic signal at the Station Avenue intersection along CR 673. Pedestrian signals with countdown timers, as well as curb ramps should be installed as part of the traffic signal installation.


Location \#4: CR 702 (Berlin Road) at station access drive
This unsignalized intersection is one of the Lindenwold station's two access drives onto CR 702. Analysis by the study team indicates that the intersection approaches are operating at an LOS A or B in the AM peak hour. In the PM peak hour, the southbound left-turn and westbound through approaches experience delays of LOS D and E due to the CR 673 traffic signals.

It is recommended that a crosswalk be striped across the station access drive in order to provide a pedestrian pathway from the station area toward the intersection of CR 702 and CR 673. This intersection is the preferred location for pedestrians to cross CR 702. The combination of the Lindenwold streetscape project and the addition of crosswalk striping at this location should make pedestrians feel more comfortable walking along Berlin Road rather than jaywalking to the businesses along the south side of the roadway.

## 人 Immediate Recommendations

- Stripe crosswalk markings across the station access drive at the intersection to make the crossing visible.

Location \#5: CR 702 (Berlin Road) at New Road
This signalized location is the main entrance and exit to the Lindenwold station. Analysis by the study team indicates that the intersection is operating at a LOS C overall in both the AM and PM peak hour. In the PM peak hour, the southbound through approach and westbound right-turn approach experience slightly higher delay with LOS D. These levels of delay are well within the preferred range and signal modifications are not necessary.

The intersection currently has pedestrian signals and curb ramps, but it is recommended that the pedestrian signals be upgraded to countdown signals as part of the overall effort to make the station area more walkable.

## © Immediate Recommendations

- Restripe crosswalk markings at the intersection to make crossings more visible.
- Install pedestrian signals with countdown timers.

Location \#6: US 30 at New Road
This complex intersection links the Lindenwold station with US 30. The existing intersection includes a jughandle from eastbound US 30, which merges with the old Bradlee's site access drive to create the southern leg of the intersection. There is a bulb-out located at the eastern corner of the intersection, in front of CVS Pharmacy. As a result, the right-turn movement from westbound US 30 to northbound New Road is restricted making it especially difficult for buses to maneuver.

There are numerous business driveways along the section of US 30 between CR 673 and New Road. This location was identified as a crash cluster link during analysis by the study team. Access management techniques, such as consolidation of driveways and installation of Right-In Right-Out Only islands, should be applied as possible at these driveways. These measures should reduce the number of angle and left-turn crashes occurring at this location.

Additionally, this intersection will be impacted by several projects currently in the planning phase. The Borough of Stratford has redevelopment plans for the old Bradlee's Site that will impact traffic at the intersection. UMDNJ has a Master Plan that includes a connector roadway between the campus and US 30 through what was a car dealership. At its intersection with US 30, the permitted movements will be right-in, right-out only.

This connector roadway will link the campus with the Lindenwold station. The study team recommends pedestrian oriented improvements along New Road between Berlin Road and US 30. These improvements and the proposed connector will make the transit options available at the station more appealing to UMDNJ faculty and students.

The study team also met with representatives from Stratford to discuss their redevelopment plans and then created two development scenarios that were used to analyze potential recommendations at the intersection. The development scenarios are discussed in detail in Appendix C.

Several potential alternatives were considered for this intersection including both removal and relocation of the New Road jughandle. Based on the LOS analysis of the potential scenarios, the study team is recommending that the jughandle be removed, left-turn lanes be added along US 30, and a southbound through/right-turn lane be added along New Road. These improvements will reduce the overall delay at the intersection and should accommodate additional trips generated by redevelopment in the area. Furthermore, the additional traffic using the intersection to access US 30 eastbound due of the recommended prohibition of turns at the CR 673 at US 30 intersection, was found to have very minimal impact on the operation of this intersection.

The recommended improvements for this intersection are illustrated on Figure 7.
© Immediate Recommendations

- Stripe crosswalk markings at the intersection to make crossings more visible.
- Install pedestrian signals with countdown timers.
- Install sidewalk along New Road between Berlin Road and US 30 to complete the pedestrian pathway between the UMDNJ connector roadway and the station.

Long-Term Recommendations

- The jughandle from eastbound US 30 to northbound New Road should be removed.
- Left-turn lanes should be installed along eastbound and westbound US 30.
- A through/right lane should be added along southbound New Road.
- The bulb-out should be removed in order to allow for a greater turning radius at the eastern corner of the intersection.
- Access management techniques should be applied as possible at the business drives throughout the section of US 30 between CR 673 and New Road.


This intersection is located at one of the two entrances into the UMDNJ campus from CR 673. This unsignalized intersection serves as the entrance to the patient complex. The university has an officer directing traffic in the AM and PM peak periods at this intersection to assist motorists in entering the CR 673 traffic stream. The crashes that are occurring along this section of CR 673 are typical for areas of congestion and no countermeasures are being recommended. The study team is recommending only that pavement markings be restriped at this intersection.

## () Immediate Recommendations

- Restripe crosswalk markings at the intersection to make them more visible.


## Location \#8: CR 673 at Medical Center Drive

This signalized intersection is the second entrance to the UMDNJ campus. The four-leg intersection consists of the two CR 673 approaches, as well as the Medical Center Drive approach and a medical complex drive as the fourth approach. This intersection was included in the CR 673 Arterial Progression Study. The recommendations from this study moved into the preliminary engineering phase, but were not implemented. It was determined that advancing the project to the design phase would result in minimal improvement due to the fact that the US 30 signal, a pivitol signal in the corridor, would not be modified as part of the project.

UMDNJ is proposing an additional entrance into the campus. This connector roadway will connect the campus parking area with US 30 at a location west of the New Road intersection. The drive will be restricted access and will be configured as a right-in-right-out (RIRO). This connector roadway will not directly connect with Medical Center Drive in an effort to prevent cutthrough traffic in the campus area. This additional entrance will alleviate some of the campus traffic along CR 673 by allowing students and faculty to use US 30 as an alternate route.
© Immediate Recommendations

- Upgrade to pedestrian signals with countdown timers.
- Restripe crosswalk markings at the intersection to make them more prominent.


## © Long-Term Recommendations

- The study team recommends that the UMDNJ connector be built as proposed, with pedestrian accommodations such as pedestrian scale lighting and streetscaping.

Location \#9/\#10: CR 686 (Gibbsboro Road) at CR 702 (Berlin Road/Egg Harbor Road)
These two closely spaced intersections lie on either side of the NJ Transit Altantic City rail line overpass. They currently experience a great deal of delay in the peak hours. In the AM peak hour, the northbound traffic along CR 686 is known to queue as far south as the White Horse Pike intersection. In the PM peak hour it is the southbound traffic that experiences large queues. The semi-actuated intersections, which are spaced less than 200 feet apart, are controlled by a single controller.

Several alternative recommendations were analyzed for these intersections. Alternative one maintains the existing geometry at the intersections but optimized the traffic signal timing. Other alternatives looked at increasing capacity by modifying the northbound left-turn lane at Berlin Road to a combination through/left lane and adding an additional lane north of the Egg Harbor Road intersection.


Northern approach of the Egg Harbor Road intersection

Due to the geographic limitations of the area, the logical extension of the additional lane terminates at Burrows Lane. However, due to the heavy through movements experienced along Gibbsboro Road, the addition of capacity has limited or no impact on the LOS. None of the time savings, in terms of delay, were determined to be worth the cost of construction. Therefore, only minor traffic control improvements are being recommended at these intersections.

人) Immediate Recommendations

- Restripe crosswalk markings at the intersections to make them more prominent.
- Upgrade to pedestrian signals with countdown timers.


Eastern approach of the Berlin Road intersection

## Transit Services

## Existing Transit Service

In addition to the PATCO high speed line to Philadelphia, the Lindenwold station is served by NJ Transit bus and the Atlantic City Rail line. The NJ Transit bus routes that lie within the study area all make stops at the Lindenwold Station.

## Passenger Rail Service

## PATCO Speedline

The Port Authority Transit Corporation (PATCO) operates the Speedline from Center City Philadelphia to New Jersey and terminates at the Lindenwold Station. Trains run 24 hours a day with the shortest headways being 4-12 minutes during weekday rush periods and as long as 45 minutes during weekday nights. Average headways are 15-20 minutes. A one-way trip from Lindenwold to Philadelphia on PATCO costs $\$ 2.70$. The average time from Philadelphia's $15^{\text {th }} / 16^{\text {th }}$ Street Station to Lindenwold is about 26 minutes.

New Jersey Transit Atlantic City Rail Line
The Atlantic City Line connects Philadelphia and Atlantic City while passing through major transit points including Cherry Hill and Lindenwold. Running to Atlantic City, service is offered beginning at 5:30 AM on weekdays and 6:00 AM on the weekends with service ending at 2:30 AM. Headways range from 45 minutes to 2 hours. A one way ticket from Lindenwold to either endpoint is about $\$ 4$. This trip ranges from 60 to 90 minutes.

## Bus Service

Four New Jersey Transit bus lines service the Lindenwold Station stop located outside the west door of the station building. These routes are the 403, 451, 459, and 554. Figure 8 illustrates the NJ Transit routes.

Route 403 connects Philadelphia and Turnersville, running along Haddon Avenue and White Horse Pike. Weekday service is offered from 6:46 AM until 2:31 AM to Turnersville and from 4:43 AM until 12:33 AM to Camden. For most of the day, headways range from 15-20 minutes. Before 8:00 AM and after 8:00 PM, headways are $45-50$ minutes long. On the weekends, longer headways are adopted, between $45-60$ minutes. Traveling between Lindenwold and Philadelphia by bus costs $\$ 3.90$ for a one-way trip and is approximately 1 hour and 25 minutes.


Route 451 runs from Camden to Lindenwold along Haddon Avenue and Haddonfield Berlin Road with weekday service only. Service begins at $5: 47$ AM and continues until 6:47 PM to Camden and from 7:17 AM until 7:22 PM to Lindenwold. Headways are 60 minutes long. The trip from Camden to Lindenwold costs $\$ 2.30$ each way and is approximately 1 hour and 20 minutes.

Route 459 connects Voorhees Town Center and the Avandale Park and Ride in Winslow Township. It runs through Lindenwold and Gloucester. During the week, service is offered from 6:19 AM until 10:54 PM to Voorhees Town Center and from 7:09 AM until 11:34 PM to Avandale. Headways during the morning rush are approximately 30 minutes and change to 60 minutes during the rest of the day. Service is abbreviated on the weekends with 1 hour and 15 minute headways, later starts and earlier finishes. The trip between Avandale and Voorhees Town Center costs $\$ 4.35$ for a one-way trip and is approximately 1 hour and 5 minutes.

Route 554 runs along US 30 (White Horse Pike) from Lindenwold to Atlantic City. This route is operated 24 hours of the day. During the week, 30 minute headways are offered during morning and afternoon rush periods. Otherwise, buses run every 60 minutes. The same service is offered on the weekends without shorter afternoon headways. The trip between Lindenwold and Atlantic City costs $\$ 5.90$ each way and is about 1 hour and 40 minutes long.

## Other Services

## Taxicabs

Taxicabs queue in a staging area outside the east door of the Lindenwold Station building. Taxicabs were observed to be waiting and available to passengers exiting the station building during all visits to the station within normal operating hours. Seven spaces have been designated in this location for taxicabs waiting for passengers. Signage instructs drivers to park in these spaces and limit themselves to seven at a time to accomplish this. Three different cab companies service the station


Taxicab staging area including Millennium, Road Runner Cab, and Lindenwold Cab.

## Routing

The Station Avenue entrance to the station is used by buses during PM Peak Hour because of delays to the schedule by using the signal at Berlin to access the New Road entrance. The installation of a traffic signal here will assist buses entering/exiting the station at this location.

Buses were observed dwelling along Berlin Road. The location of this dwell could be relocated to DRPA/PATCO property in the farthest east parking area in order to remove the buses from the shoulder area along Berlin Road.

Transit recommendations include the following:
人 Immediate Recommendations

* Remove the two "Speed Hump" signs in front of the station. There are no speed humps in these locations.
- Relocate the bus dwell area to the station property.

人 Long-Term Recommendations

- Install a covered walkway or canopy from the station building to the bus stop area. This will increase the sheltered waiting area and provide a covered walkway to the station building. An additional advantage will be a covered bicycle rack area on the west side of the station.


NJ Transit bus dwelling along CR 702

## Bicycle and Pedestrian Facilities

## Safety and Accessibility

Americans with Disabilities Act (ADA) compliant facilities are provided within the PATCO station building. However, the building is ADA accessible through the west end of the building only. The handicapped parking area crosswalk leads to the center door, which then connects to the ramp inside the station building. There is an elevator that leads to the platform once through the fare control machines.

The Atlantic City rail line is accessible two ways, through the PATCO station building or through a tunnel located just east of the station building. NJ Transit ticket machines are located in a shelter located on the platform for the Atlantic City rail line along the rear of the PATCO station. The pedestrian tunnel at the east end of the platform provides additional sheltered waiting area.


Lindenwold Platform on the Atlantic City Rail Line

There is one bus stop shelter at the station, located at the west end of the station building. During several visits to the station, the number of people waiting at the stop was more than could be accommodated within the existing shelter or on the adjacent benches. During times of inclement weather, a large number of people were observed waiting inside the station building. The Increasing Intermodal Access to Transit study recommended that pedestrian striping and signage be added within the station parking area to enhance safety for pedestrians when vehicles are entering and exiting the lots.

The study team makes the following recommendations in order to improve accessibility in and around the station. These improvements will create a more pedestrian and bicycle friendly environment that will impact all users of the station by creating a safer route between parking areas and the station.


NJ Transit bus stop area

The recommended improvements for the station area are illustrated on Figure 9.

## Immediate Recommendations

- A. Install a raised crosswalk in the designated crossing area leading from the handicap parking area to the door at the center of the station.
B. Add curb ramps and crosswalks at all of the internal intersections and drive crossings.
- C. Create a pedestrian pathway from the west end of the station building to the edge of the south parking lot at Berlin Road.
- Extend the existing crosswalk through the drive aisles in the south parking lot.
- Install a raised crosswalk across the internal station drive between the paid parking lot and the south lot.
- D. Add parking blocks at parking spaces that front the major pedestrian pathways to prevent vehicles from overhanging the sidewalk.
- Install a raised intersection where the station access drive from Berlin Road crosses the Station Avenue extension drive.


## 人 Long-Term Recommendations

- D. Relocate signing and light poles as feasible to eliminate sidewalk obstructions.
- E. Create a designated pedestrian pathway from the station toward the UMDNJ connector by installing a raised crosswalk at the east edge of the taxicab area.

A. Raise the crosswalk between the station and the handicap parking

B. Provide curb ramps within the station parking

C. Emphasize the crossing between the paid parking area and the south lot

D. Relocate signs and light poles, and install parking blocks

E. Install a raised crosswalk along the east edge of the taxicab staging area



## Pedestrian Connectivity

The station has poor pedestrian connectivity with the surrounding area. While there are sidewalks within the station area, there are no sidewalks along Berlin Road or Laurel Road to connect the station or the businesses south of the station. There is a sidewalk along the northwest corner of the station along CR 673 that leads to Voorhees Township. However, this sidewalk is connected to the station via a stairway that is inaccessible to bicycles, baby strollers, and wheel chairs. Additionally, the CR 673 bridge over the railroad has sidewalk only on the station side of the street; signage prohibits pedestrians on the northwest side of the roadway.

There are no midblock crossings between the station and the businesses along Berlin Road. A number of pedestrians were observed jaywalking across Berlin Road to the businesses along the south side of the roadway during various site visits.

Pedestrian Level of Service (PLOS) results from the DVRPC study conducted in $2005^{1}$ revealed that many of the residential streets within a one-quarter mile radius of the station are considered LOS B while CR 673 is LOS C and Berlin Road is LOS D. In order to improve the quality of the pedestrian facilities surrounding that station, it was recommended that sidewalks be added along Berlin Road and CR 673 and that pedestrian signals and striping be added at the intersection of Berlin with CR 673. Additionally, it was recommended that the intersection of CR 673 and Station Avenue be signalized with pedestrian actuation.

The Borough of Lindenwold has received funding from SAFETEA-LU for streetscape and pedestrian improvements along Berlin road. These improvements will include street trees, benches, and lighting and will run the length of Berlin Road and connect the station with the Gibbsboro Road corridor. The DVRPC study team recommends that these type of pedestrian friendly amenities be continued throughout the study area, especially along the walkway between the station and the proposed UMDNJ connector.

Trees should be planted along buffer areas between pedestrians and vehicles and street furniture such as benches should be placed to accommodate pedestrians. Pedestrian level lighting should also be provided to create a sense of security among pedestrians. These types of features should coordinate with those being placed by the Lindenwold SAFETEA-LU funded streetscape project.

UMDNJ is promoting the use of transit to its students and faculty, but is currently disconnected from the station due to missing sections of sidewalk. At present the path to the university is marked by dirt pathways and stepping stone blocks along CR 673 from Berlin Road to Central Avenue. The current walk to the UMDNJ campus is approximately 0.6 mile, which will be reduced to approximately 0.5 mile once the UMDNJ connector roadway is constructed. These walkways, as well as the sidewalk conditions are illustrated on Figure 10. These pedestrian pathways should be designated by constructing sidewalk along with other amenities as discussed above.

[^0]The proposed recommendations will make existing connections, as well as the use of public transit more appealing.

人 Immediate Recommendations

- Install pedestrian signals with countdown timers and highly visible crosswalk markings at signalized intersections to provide pedestrians with safer crossings.
- Designate a pedestrian pathway along internal station roadways from the station building and along Berlin Road to the signalized intersections.
- Along Berlin Road to the signalized intersection at CR 673, then along CR 673 to UMDNJ.
- Across Berlin Road at the New Road signalized intersection and along New Road across White Horse Road to the proposed UMDNJ connector.
- Add sidewalk along the perimeter of the PATCO property abutting CR 673.


## $\Leftrightarrow$ Long-Term Recommendations

- Integrate street trees, lighting, and furniture such as benches along proposed sidewalks. This is especially important in the area between the station and the proposed UMDNJ connector.


## Bicycle Connectivity

Currently there is a multi-use trail along Berlin Road between Linden Avenue and Gibbsboro Road. Additionally, there is a bicycle lane along Berlin Road between New Road and Linden Avenue that connects to the trail at Linden Avenue. This bicycle network does not provide a direct connection to the station.

DVRPC's Transit, Bicycle and Pedestrian Planning unit recently conducted the Central Camden County Bicycling \& Multi-Use Trails Master Plan. This plan included the area surrounding the Lindenwold station.
Recommendations for the area include a 'recommended route' to link the residential areas with the station. This proposed route will also connect to the existing bicycle lane along Berlin Road and the proposed redevelopment area at the New Road extension. The plan does not propose any changes to the roadways; they are bicycle friendly as they are. Improvements such as


Bicycle racks at the Lindenwold Station wayfinding and "Share The
Road" signage are recommended for these routes. The addition of a traffic signal at the intersection of CR 673 and Station Avenue would greatly improve the safety of this recommended route by providing a signalized crossing for both pedestrians and bicyclists.

The following improvements, along with those mentioned in the preceding sections, will enhance the station's connectivity and make the use of public transit more appealing to a larger group of people.

人 Immediate Recommendations

- Wayfinding and 'Share The Road' signage should be considered throughout the study area, especially along the internal station roadway between Station Avenue and New Road.
- Explore the feasibility of installing an accessible ramp alongside the stairway between CR 673 and the station. If this isn't determined to be feasible, a pedestrian friendly pathway along the Station Avenue entrance to the station can serve as the alternative for strollers and bicycles.
- Bicycle racks should be monitored to remove stripped or abandoned bicycle frames.



## Implementation

This report's recommendations aim to provide access and connectivity to the Lindenwold Station, in turn providing mobility to the eastern portion of Camden County. The implementation of these recommendations relies upon the study area municipalities and agencies. This section summarizes each recommendation by subsection, estimates possible project costs, and identifies the responsible agency.

The following estimates for Immediate Recommendations are for construction costs only. Estimates for Long-Term Recommendations include preliminary engineering, final design, and construction costs and are based on similar projects currently listed on the DVRPC Transportation Improvement Program (TIP).

Tables 5-7 chart the estimated costs and responsible agencies for the improvements outlined in the report.

## Intersection Recommendations

Table 5: Intersection Recommendations

| Recommendation |
| :--- |
| Location \#1/\#2: CR 673 at CR 702/US 30 |
| Installation of countdown pedestrian signals |
| and curb ramps |
| Installation of crosswalk and "Don't Block the |
| Box" markings |
| Radii and curb modification |
| Location \#3: CR 673 at Station Avenue |
| Installation of crosswalk and "Don't Block the |
| Box" markings |
| Extension of CR 673 NB curb lane to station |
| Installation of traffic signal with countdown |
| pedestrian signals and curb ramps |

Responsible Agency
Camden County
Stratford Municipal
Officials
NJDOT

DRPA/ PATCO
Camden County
Somerdale Municipal
Officials
NJDOT

## Project Cost Estimate

\$200,000 - \$250,000 per intersection

## \$4,000-\$6,000 per intersection

\$20,000 - \$25,000
\$4,000-\$6,000 per intersection
\$20,000 - \$25,000
\$80,000 - \$100,000

Table 5: Intersection Recommendations (Continued)

| Recommendation | Responsible Agency | Project Cost Estimate |
| :--- | :--- | :--- |
| Locations \#4/\#5: CR 702 at station <br> access/New Road <br> Installation of countdown pedestrian signals <br> and curb ramps | DRPA/ PATCO <br> Camden County <br> Stratford Municipal <br> Officials <br> NJDOT | $\$ 200,000-\$ 250,000$ per intersection |

## Transit Accessibility Recommendations

## Table 6: Transit Recommendations

| Recommendation |
| :--- |
| Removal of incorrect speed hump signage |
| Relocation of bus dwell location |
| Installation of a covered walkway or canopy |
| S ource: DVRPC 2009 |

Responsible Agency
NJ Transit
PATCO

Project Cost Estimate
Minimal cost to agency
Minimal cost to agency
\$40,000 - \$58,000

## Bicycle and Pedestrian Facility Recommendations

Table 7: Bicycle and Pedestrian Recommendations

| Recommendation | Responsible Agency | Project Cost Estimate |
| :--- | :--- | :--- |
| Installation of crosswalk markings and curb <br> ramps at internal intersections and drives | DRPA <br> PATCO | $\$ 2,000-\$ 3,000$ per location |
| Installation of raised crosswalks (3) |  | $\$ 4,000-\$ 8,000$ Per location |
| Installation of a raised intersection | $\$ 15,000-\$ 20,000$ |  |
| Addition of parking blocks | Minimal cost to agency |  |
| Relocation of sign posts and light poles | $\$ 250-\$ 350$ per sign |  |
| Installation of sidewalk - CR 673 | $\$ 2,500-\$ 4,000$ per pole |  |
| Integration of street trees, lighting, and |  |  |
| furniture |  | $\$ 18,000-\$ 24,000$ (between station drive |
| Installation of wayfinding and "Share The 702$)$ |  |  |
| Road" signage | $\$ 2,500-\$ 4,000$ per item |  |
| Explore the feasibility of a ramp along the CR |  |  |
| 673 stairway | $\$ 1,000-\$ 1,500$ per sign |  |

Source: DVRPC 2009

DVRPC, Central Camden County Bicycling \& Multi-Use Trails Master Plan, July 2009
DVRPC, Increasing Intermodal Access to Transit, Phase 2, June 2005
DVRPC, Survey Support for PATCO Transit Extension Study, February, 2008
Environmental Resolutions for Borough of Lindenwold, Berlin Road Streetscape Project, 2009
Institute of Transportation Engineers, Trip Generation, 7th Edition, 2003
Orth-Rodgers for DVRPC/DRPA, PATCO Parking Study, December 2002
Remington \& Vernick Engineers for DVRPC, Feasibility Study: Arterial Progression County Road 673 (Laurel Road), December 2005

Wallace Roberts \& Todd for DRPA, PATCO Transit Oriented Development Master Plans Study, July 2006

APPENDIX A

## Traffic Data

## Traffic Volume Data

Raw traffic volume data for the intersections where DVRPC collected data are shown on the following pages, Figures A-1 through A-13.

## System Peak Hour

Peak hour turning movement data and system peak hour tabulations are shown in Tables A-1 and A-2 found on pages A-22 and A-23.

## Signal Warrant

The signal warrant for the Station Avenue intersection with CR 673 is shown in Figures A-14 and A-15 found on pages A-24 through A-27.

## 



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Figure A-1: Traffic Volume Data - CR 673 at CR 702 (Continued)
DELAWARE VALLEY REGIONAL PLANNING COMMISSION
OFFICE OF TRAVEL MAM INTERVAL COUNTS $\begin{array}{ll}\text { CAMDEN } & \\ \text { STRATFORD } & \\ & \\ \text { North-South Street } & \text { \& } \\ \text { LAUREL RD / WHITE HORSE RD } & \text { East-West Street } \\ \end{array}$

| STARTING <br> TIME | LAUREL RD / WHITE HORSE RD |  |  |  |  |  |  |  |  | CR 702 BERLIN RD |  |  |  |  |  |  | N-S | E-W | TOTAL |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 1-NORTHBOUND |  |  |  | 2-SOUTHBOUND |  |  |  | 3-EASTBOUND |  |  |  | 4-WESTBOUND |  |  |  |  |  |
|  | L | S | R | TOTAL | L | S | R | TOTAL | L | S | R | TOTAL | L | S | R | TOTAL | TOTAL | TOTAL |  |
| 3:00 3:15 | 5 | 144 | 13 | 162 | 69 | 127 | 16 | 212 | 3 | 6 | 11 | 20 | 27 | 33 | 52 | 112 | 374 | 132 | 506 |
| 3:15 3:30 | 0 | 117 | 15 | 132 | 60 | 120 | 16 | 196 | 6 | 3 | 2 | 11 | 27 | 41 | 60 | 128 | 328 | 139 | 467 |
| 3:30 3:45 | 2 | 155 | 12 | 169 | 63 | 87 | 28 | 178 | 4 | 9 | 5 | 18 | 26 | 57 | 65 | 148 | 347 | 166 | 513 |
| 3:45 4:00 | 2 | 138 | 8 | 148 | 82 | 127 | 15 | 224 | 5 | 4 | 9 | 18 | 16 | 56 | 58 | 130 | 372 | 148 | 520 |
| 4:00 4:15 | 2 | 154 | 16 | 172 | 75 | 115 | 14 | 204 | 6 | 4 | 14 | 24 | 27 | 65 | 79 | 171 | 376 | 195 | 571 |
| 4:15 4:30 | 1 | 128 | 12 | 141 | 82 | 117 | 10 | 209 | 8 | 3 | 7 | 18 | 36 | 44 | 67 | 147 | 350 | 165 | 515 |
| 4:30 4:45 | 3 | 115 | 11 | 129 | 67 | 99 | 13 | 179 | 5 | 9 | 10 | 24 | 34 | 59 | 50 | 143 | 308 | 167 | 475 |
| 4:45 5:00 | 0 | 163 | 12 | 175 | 83 | 108 | 11 | 202 | 3 | 6 | 6 | 15 | 38 | 52 | 74 | 164 | 377 | 179 | 556 |
| 5:00 5:15 | 1 | 148 | 13 | 162 | 83 | 80 | 12 | 175 | 25 | 6 | 15 | 46 | 50 | 75 | 59 | 184 | 337 | 230 | 567 |
| 5:15 5:30 | 1 | 124 | 12 | 137 | 95 | 106 | 8 | 209 | 7 | 6 | 16 | 29 | 41 | 47 | 63 | 151 | 346 | 180 | 526 |
| 5:30 5:45 | 1 | 129 | 10 | 140 | 114 | 120 | 13 | 247 | 6 | 6 | 7 | 19 | 34 | 85 | 66 | 185 | 387 | 204 | 591 |
| 5:45 6:00 | 0 | 96 | 14 | 110 | 107 | 120 | 4 | 231 | 5 | 3 | 6 | 14 | 45 | 70 | 45 | 160 | 341 | 174 | 515 |
| 6:00 6:15 | 0 | 91 | 20 | 111 | 85 | 115 | 3 | 203 | 9 | 5 | 6 | 20 | 35 | 57 | 48 | 140 | 314 | 160 | 474 |
| 6:15 6:30 | 2 | 96 | 12 | 110 | 102 | 112 | 12 | 226 | 6 | 7 | 7 | 20 | 34 | 50 | 46 | 130 | 336 | 150 | 486 |
| 6:30 6:45 | 1 | 93 | 14 | 108 | 101 | 105 | 10 | 216 | 7 | 4 | 6 | 17 | 33 | 54 | 43 | 130 | 324 | 147 | 471 |
| 6:45 7:00 | 0 | 85 | 10 | 95 | 94 | 107 | 7 | 208 | 5 | 5 | 4 | 14 | 31 | 51 | 47 | 129 | 303 | 143 | 446 |
| TOTALS | 21 | 1976 | 204 | 2201 | 1362 | 1765 | 192 | 3319 | 110 | 86 | 131 | 327 | 534 | 896 | 922 | 2352 | 5520 | 2679 | 8199 |
| Peak Hour totals |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4:45-5:45 pm | 3 | 564 | 47 |  | 375 | 414 | 44 |  | 41 | 24 | 44 |  | 163 | 259 | 262 |  |  |  | 2240 |
| PHF | 0.75 | 0.87 | 0.90 |  | 0.82 | 0.86 | 0.85 |  | 0.41 | 1.00 | 0.69 |  | 0.82 | 0.76 | 0.89 |  |  |  |  |
| System 4:30-5:30 pm | 5 | 550 | 48 |  | 328 | 393 | 44 |  | 40 | 27 | 47 |  | 163 | 233 | 246 |  |  |  | 2124 |
| PHF | 0.42 | 0.84 | 0.92 |  | 0.86 | 0.91 | 0.85 |  | 0.40 | 0.75 | 0.73 |  | 0.82 | 0.78 | 0.83 |  |  |  |  |

Figure A-2: Traffic Volume Data - CR 673 at US 30


## 

Figure A-2: Traffic Volume Data - CR 673 at US 30 (Continued)
DELAWARE VALLEY REGIONAL PLANNING COMMISSION
OFFICE OF TRAVEL M AM INTERVAL COUNTS East-West Street
US 30 WHITE HORSE PIKE

| M | CR 673 LAUREL RD |  |  |  |  |  |  |  |  | US 30 WHITE HORSE PIKE |  |  |  |  |  |  |  |  | TOTAL |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| STARTING | 1-NORTHBOUND |  |  |  |  | 2-SOUTHBOUND |  |  |  | 3-EASTBOUND |  |  |  | 4-WESTBOUND |  |  | N-S | E-W |  |
| TIME | L | S | R | TOTAL | L | S | R | TOTAL | L | S | R | TOTAL | L | S | R | TOTAL | TOTAL | TOTAL |  |
| 3:00 3:15 | 39 | 160 | 3 | 202 | 13 | 130 | 5 | 148 | 1 | 234 | 24 | 259 | 0 | 191 | 18 | 209 | 350 | 468 | 818 |
| 3:15 3:30 | 27 | 129 | 7 | 163 | 15 | 133 | 2 | 150 | 0 | 263 | 28 | 291 | 0 | 210 | 18 | 228 | 313 | 519 | 832 |
| 3:30 3:45 | 46 | 146 | 2 | 194 | 15 | 123 | 0 | 138 | 0 | 284 | 36 | 320 | 0 | 163 | 21 | 184 | 332 | 504 | 836 |
| 3:45 4:00 | 26 | 148 | 2 | 176 | 14 | 116 | 3 | 133 | 0 | 297 | 29 | 326 | 0 | 182 | 25 | 207 | 309 | 533 | 842 |
| 4:00 4:15 | 48 | 159 | 1 | 208 | 21 | 130 | 7 | 158 | 0 | 276 | 33 | 309 | 0 | 203 | 21 | 224 | 366 | 533 | 899 |
| 4:15 4:30 | 34 | 177 | 1 | 212 | 9 | 125 | 4 | 138 | 1 | 350 | 29 | 380 | 0 | 243 | 19 | 262 | 350 | 642 | 992 |
| 4:30 4:45 | 57 | 169 | 0 | 226 | 10 | 141 | 6 | 157 | 0 | 319 | 38 | 357 | 0 | 181 | 15 | 196 | 383 | 553 | 936 |
| 4:45 5:00 | 46 | 159 | 0 | 205 | 12 | 112 | 3 | 127 | 0 | 332 | 35 | 367 | 0 | 209 | 22 | 231 | 332 | 598 | 930 |
| 5:00 5:15 | 59 | 156 | 2 | 217 | 10 | 144 | 5 | 159 | 0 | 340 | 32 | 372 | 0 | 189 | 16 | 205 | 376 | 577 | 953 |
| 5:15 5:30 | 50 | 144 | 0 | 194 | 11 | 138 | 5 | 154 | 0 | 341 | 27 | 368 | 1 | 220 | 19 | 240 | 348 | 608 | 956 |
| 5:30 5:45 | 27 | 136 | 1 | 164 | 20 | 146 | 2 | 168 | 0 | 319 | 28 | 347 | 0 | 184 | 24 | 208 | 332 | 555 | 887 |
| 5:45 6:00 | 33 | 131 | 1 | 165 | 16 | 126 | 5 | 147 | 0 | 339 | 39 | 378 | 0 | 172 | 23 | 195 | 312 | 573 | 885 |
| 6:00 6:15 | 30 | 121 | 2 | 153 | 19 | 142 | 1 | 162 | 0 | 305 | 31 | 336 | 0 | 184 | 19 | 203 | 315 | 539 | 854 |
| 6:15 6:30 | 34 | 132 | 0 | 166 | 11 | 125 | 3 | 139 | 0 | 271 | 30 | 301 | 0 | 188 | 29 | 217 | 305 | 518 | 823 |
| 6:30 6:45 | 26 | 120 | 4 | 150 | 12 | 130 | 2 | 144 | 0 | 245 | 31 | 276 | 1 | 182 | 19 | 202 | 294 | 478 | 772 |
| 6:45 7:00 | 23 | 121 | 0 | 144 | 10 | 137 | 2 | 149 | 0 | 264 | 34 | 298 | 0 | 159 | 16 | 175 | 293 | 473 | 766 |
| TOTALS | 605 | 2308 | 26 | 2939 | 218 | 2098 | 55 | 2371 | 2 | 4779 | 504 | 5285 | 2 | 3060 | 324 | 3386 | 5310 | 8671 | 13981 |
| Peak Hour totals |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4:15-5:15 pm | 196 | 661 | 3 |  | 41 | 522 | 18 |  | 1 | 1341 | 134 |  | 0 | 822 | 72 |  |  |  | 3811 |
| PHF | 0.83 | 0.93 | 0.38 |  | 0.85 | 0.91 | 0.75 |  | 0.25 | 0.96 | 0.88 |  | 0.00 | 0.85 | 0.82 |  |  |  |  |
| System 4:30-5:30 pm | 212 | 628 | 2 |  | 43 | 535 | 19 |  | 0 | 1332 | 132 |  | 1 | 799 | 72 |  |  |  | 3775 |
| PHF | 0.90 | 0.93 | 0.25 |  | 0.90 | 0.93 | 0.79 |  | 0.00 | 0.98 | 0.87 |  | 0.25 | 0.91 | 0.82 |  |  |  |  |

Figure A-3: Traffic Volume Data - CR 673 at Station Avenue
DELAWARE VALLEY REGIONAL PLANNING COMMISSION
OFFICE OF TRAVEL M AM INTERVAL COUNTS East-West Street
WHITE HORSE RD
ぁ

F

8

> CAMDEN
SOMERDALE North-South Street
STATION AVE 1/13/09
TUESDAY
FAIR

3PM

|  |  |  |  |  | STATION |  |  |  |  |  |  |  | WHITE H | RE R |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| STARTING |  |  | 1-NO | THBO |  |  | 2-SO | THBO |  |  |  | TBO |  |  | 4-W | STBO |  | N-S | E-W |  |
| TIME |  | L | S | R | TOTAL | L | S | R | TOTAL | L | S | R | TOTAL | L | S | R | TOTAL | TOTAL | TOTAL | TOTAL |
| 3:00 3:15 |  | 0 | 0 | 7 | 7 | 0 | 0 | 0 | 0 | 0 | 122 | 2 | 124 | 28 | 166 | 2 | 196 | 7 | 320 | 327 |
| 3:15 3:30 |  | 1 | 0 | 11 | 12 | 2 | 0 | 2 | 4 | 0 | 148 | 2 | 150 | 36 | 157 | 2 | 195 | 16 | 345 | 361 |
| 3:30 3:45 |  | 1 | 0 | 12 | 13 | 1 | 1 | 0 | 2 | 0 | 156 | 4 | 160 | 52 | 182 | 3 | 237 | 15 | 397 | 412 |
| 3:45 4:00 |  | 0 | 0 | 17 | 17 | 1 | 0 | 0 | 1 | 0 | 182 | 1 | 183 | 47 | 191 | 2 | 240 | 18 | 423 | 441 |
| 4:00 4:15 |  | 0 | 0 | 15 | 15 | 1 | 0 | 1 | 2 | 0 | 178 | 3 | 181 | 53 | 190 | 3 | 246 | 17 | 427 | 444 |
| 4:15 4:30 |  | 0 | 0 | 14 | 14 | 2 | 1 | 0 | 3 | 0 | 170 | 6 | 176 | 64 | 196 | 3 | 263 | 17 | 439 | 456 |
| 4:30 4:45 |  | 1 | 0 | 29 | 30 | 3 | 0 | 1 | 4 | 0 | 181 | 5 | 186 | 58 | 198 | 2 | 258 | 34 | 444 | 478 |
| 4:45 5:00 |  | 0 | 0 | 26 | 26 | 2 | 1 | 3 | 0 | 0 | 190 | 8 | 198 | 78 | 196 | 2 | 276 | 26 | 474 | 500 |
| 5:00 5:15 |  | 1 | 0 | 34 | 35 | 2 | 0 | 1 | 3 | 0 | 190 | 9 | 199 | 75 | 208 | 2 | 285 | 38 | 484 | 522 |
| 5:15 5:30 |  | 1 | 0 | 24 | 25 | 1 | 0 | 0 | 1 | 0 | 138 | 4 | 142 | 63 | 184 | 4 | 251 | 26 | 393 | 419 |
| 5:30 5:45 |  | 1 | 0 | 44 | 45 | 2 | 0 | 1 | 3 | 0 | 142 | 2 | 144 | 66 | 218 | 2 | 286 | 48 | 430 | 478 |
| 5:45 6:00 |  | 1 | 0 | 40 | 41 | 0 | 0 | 1 | 1 | 2 | 95 | 2 | 99 | 42 | 184 | 8 | 234 | 42 | 333 | 375 |
| 6:00 6:15 |  | 3 | 0 | 50 | 53 | 1 | 0 | 2 | 3 | 0 | 160 | 8 | 168 | 56 | 226 | 11 | 293 | 56 | 461 | 517 |
| 6:15 6:30 |  | 1 | 0 | 44 | 45 | 0 | 0 | 1 | 1 | 0 | 160 | 5 | 165 | 37 | 213 | 13 | 263 | 46 | 428 | 474 |
| 6:30 6:45 |  | 0 | 6 | 35 | 41 | 2 | 0 | 7 | 9 | 0 | 130 | 2 | 132 | 26 | 164 | 6 | 196 | 50 | 328 | 378 |
| 6:45 7:00 |  | 1 | 0 | 22 | 23 | 1 | 0 | 2 | 3 | 0 | 114 | 1 | 115 | 30 | 142 | 7 | 179 | 26 | 294 | 320 |
|  | TOTALS | 12 | 6 | 424 | 442 | 21 | 3 | 22 | 40 | 2 | 2456 | 64 | 2522 | 811 | 3015 | 72 | 3898 | 482 | 6420 | 6902 |
|  | Peak Hour totals |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4:15-5:15 pm |  | 2 | 0 | 103 |  | 9 | 2 | 5 |  | 0 | 731 | 28 |  | 275 | 798 | 9 |  |  |  | 1956 |
| PHF |  | 0.50 | 0.00 | 0.76 |  | 0.75 | 0.50 | 0.42 |  | 0.00 | 0.96 | 0.78 |  | 0.88 | 0.96 | 0.75 |  |  |  |  |
| 4:30-5:30 pm |  | 3 | 0 | 113 |  | 8 | 1 | 5 |  | 0 | 699 | 26 |  | 274 | 786 | 10 |  |  |  | 1919 |
| PHF |  | 0.75 | 0.00 | 0.83 |  | 0.67 | 0.25 | 0.42 |  | 0.00 | 0.92 | 0.72 |  | 0.88 | 0.94 | 0.63 |  |  |  |  |

Figure A-4: Traffic Volume Data - CR 702 at station access drive

[^1]East-West Street
STATION ENTRANCE

| STARTING <br> TIME | BERLIN RD |  |  |  |  | STATION ENTRANCE |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1-NORTHBOUND |  |  |  |  | 2-SOUTHBOUND |  |  |  | 3-EASTBOUND |  |  |  | 4-WESTBOUND |  |  | N-S | E-W |  |
|  | L | S | R | TOTAL | L | S | R | TOTAL | L | S | R | TOTAL | L | S | R | TOTAL | TOTAL | TOTAL | TOTAL |
| 6:00 6:15 | 0 | 21 | 12 | 33 | 11 | 6 | 0 | 17 | 0 | 0 | 0 | 0 | 4 | 0 | 5 | 9 | 50 | 9 | 59 |
| 6:15 6:30 | 0 | 44 | 24 | 68 | 20 | 8 | 0 | 28 | 0 | 0 | 0 | 0 | 7 | 0 | 11 | 18 | 96 | 18 | 114 |
| 6:30 6:45 | 0 | 113 | 31 | 144 | 14 | 12 | 0 | 26 | 0 | 0 | 0 | 0 | 2 | 0 | 6 | 8 | 170 | 8 | 178 |
| 6:45 7:00 | 0 | 144 | 45 | 189 | 23 | 23 | 0 | 46 | 0 | 0 | 0 | 0 | 3 | 0 | 9 | 12 | 235 | 12 | 247 |
| 7:00 7:15 | 0 | 52 | 26 | 78 | 16 | 12 | 0 | 28 | 0 | 0 | 0 | 0 | 6 | 0 | 7 | 13 | 106 | 13 | 119 |
| 7:15 7:30 | 0 | 65 | 49 | 114 | 18 | 11 | 0 | 29 | 0 | 0 | 0 | 0 | 2 | 0 | 11 | 13 | 143 | 13 | 156 |
| 7:30 7:45 | 0 | 159 | 74 | 233 | 18 | 14 | 0 | 32 | 0 | 0 | 0 | 0 | 1 | 0 | 12 | 13 | 265 | 13 | 278 |
| 7:45 8:00 | 0 | 160 | 41 | 201 | 9 | 15 | 0 | 24 | 0 | 0 | 0 | 0 | 6 | 0 | 5 | 11 | 225 | 11 | 236 |
| 8:00 8:15 | 0 | 186 | 99 | 285 | 7 | 37 | 0 | 44 | 0 | 0 | 0 | 0 | 4 | 0 | 15 | 19 | 329 | 19 | 348 |
| 8:15 8:30 | 0 | 141 | 66 | 207 | 8 | 17 | 0 | 25 | 0 | 0 | 0 | 0 | 7 | 0 | 16 | 23 | 232 | 23 | 255 |
| 8:30 8:45 | 0 | 212 | 50 | 262 | 12 | 25 | 0 | 37 | 0 | 0 | 0 | 0 | 5 | 0 | 4 | 9 | 299 | 9 | 308 |
| 8:45 9:00 | 0 | 187 | 62 | 249 | 9 | 19 | 0 | 28 | 0 | 0 | 0 | 0 | 6 | 0 | 11 | 17 | 277 | 17 | 294 |
|  | 0 | 1484 | 579 | 2063 | 165 | 199 | 0 | 364 | 0 | 0 | 0 | 0 | 53 | 0 | 112 | 165 | 2427 | 165 | 2592 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 8:00-9:00 am | 0 | 726 | 277 |  | 36 | 98 | 0 |  | 0 | 0 | 0 |  | 22 | 0 | 46 |  |  |  | 1205 |
| PHF | 0.00 | 0.86 | 0.70 |  | 0.75 | 0.66 | 0.00 |  |  |  |  |  | 0.79 | 0.00 | 0.72 |  |  |  |  |
| 7:45-8:45 am | 0 | 699 | 256 |  | 36 | 94 | 0 |  | 0 | 0 | 0 |  | 22 | 0 | 40 |  |  |  | 1147 |
| PHF | 0.00 | 0.82 | 0.65 |  | 0.75 | 0.64 | 0.00 |  |  |  |  |  | 0.79 | 0.00 | 0.63 |  |  |  |  |

DELAWARE VALLEY REGIONAL PLANNING COMMISSION
OFFICE OF TRAVEL M AM INTERVAL COUNTS

## East-West Street STATION ENTRANCE

8

| STARTING | 1-NORTHBOUND ${ }^{\text {BERLIN RD }}$ |  |  |  |  | STATION ENTRANCE |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  | 2-SOUTHBOUND |  |  |  | 3-EASTBOUND |  |  |  | 4-WESTBOUND |  |  | N-S | E-W |  |
| TIME | L | S | R | TOTAL | L | S | R | TOTAL | L | S | R | TOTAL | L | S | R | TOTAL | TOTAL | TOTAL | TOTAL |
| 3:00 3:15 | 0 | 83 | 3 | 86 | 2 | 89 | 0 | 91 | 0 | 0 | 0 | 0 | 10 | 0 | 16 | 26 | 177 | 26 | 203 |
| 3:15 3:30 | 0 | 72 | 0 | 72 | 3 | 67 | 0 | 70 | 0 | 0 | 0 | 0 | 9 | 0 | 18 | 27 | 142 | 27 | 169 |
| 3:30 3:45 | 0 | 92 | 2 | 94 | 2 | 82 | 0 | 84 | 0 | 0 | 0 | 0 | 16 | 0 | 31 | 47 | 178 | 47 | 225 |
| 3:45 4:00 | 0 | 77 | 3 | 80 | 6 | 64 | 0 | 70 | 0 | 0 | 0 | 0 | 22 | 0 | 22 | 44 | 150 | 44 | 194 |
| 4:00 4:15 | 0 | 68 | 2 | 70 | 1 | 55 | 0 | 56 | 0 | 0 | 0 | 0 | 18 | 0 | 7 | 25 | 126 | 25 | 151 |
| 4:15 4:30 | 0 | 41 | 0 | 41 | 0 | 27 | 0 | 27 | 0 | 0 | 0 | 0 | 23 | 0 | 19 | 42 | 68 | 42 | 110 |
| 4:30 4:45 | 0 | 68 | 0 | 68 | 0 | 32 | 0 | 32 | 0 | 0 | 0 | 0 | 29 | 0 | 34 | 63 | 100 | 63 | 163 |
| 4:45 5:00 | 0 | 71 | 1 | 72 | 0 | 58 | 0 | 0 | 0 | 0 | 0 | 0 | 27 | 0 | 28 | 55 | 72 | 55 | 127 |
| 5:00 5:15 | 0 | 84 | 0 | 84 | 2 | 41 | 0 | 43 | 0 | 0 | 0 | 0 | 24 | 0 | 34 | 58 | 127 | 58 | 185 |
| 5:15 5:30 | 0 | 69 | 0 | 69 | 0 | 60 | 0 | 60 | 0 | 0 | 0 | 0 | 40 | 0 | 33 | 73 | 129 | 73 | 202 |
| 5:30 5:45 | 0 | 100 | 0 | 100 | 0 | 50 | 0 | 50 | 0 | 0 | 0 | 0 | 64 | 0 | 78 | 142 | 150 | 142 | 292 |
| 5:45 6:00 | 0 | 80 | 1 | 81 | 3 | 48 | 0 | 51 | 0 | 0 | 0 | 0 | 29 | 0 | 58 | 87 | 132 | 87 | 219 |
| 6:00 6:15 | 0 | 92 | 2 | 94 | 5 | 62 | 0 | 67 | 0 | 0 | 0 | 0 | 44 | 0 | 36 | 80 | 161 | 80 | 241 |
| 6:15 6:30 | 0 | 125 | 1 | 126 | 0 | 100 | 0 | 100 | 0 | 0 | 0 | 0 | 26 | 0 | 33 | 59 | 226 | 59 | 285 |
| 6:30 6:45 | 0 | 175 | 0 | 175 | 2 | 79 | 0 | 81 | 0 | 0 | 0 | 0 | 24 | 0 | 27 | 51 | 256 | 51 | 307 |
| 6:45 7:00 | 0 | 145 | 2 | 147 | 1 | 68 | 0 | 69 | 0 | 0 | 0 | 0 | 17 | 0 | 22 | 39 | 216 | 39 | 255 |
|  | 0 | 1442 | 17 | 1459 | 27 | 982 | 0 | 951 | 0 | 0 | 0 | 0 | 422 | 0 | 496 | 918 | 2410 | 918 | 3328 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6:00-7:00 pm | 0 | 537 | 5 |  | 8 | 309 | 0 |  | 0 | 0 | 0 |  | 111 | 0 | 118 |  |  |  | 1088 |
| PHF | 0.00 | 0.77 | 0.63 |  | 0.40 | 0.77 | 0.00 |  |  |  |  |  | 0.63 | 0.00 | 0.82 |  |  |  |  |
| 4:30-5:30 pm | 0 | 292 | 1 |  | 2 | 191 | 0 |  | 0 | 0 | 0 |  | 120 | 0 | 129 |  |  |  | 677 |
| PHF | 0.00 | 0.87 | 0.25 |  | 0.25 | 0.80 | 0.00 |  |  |  |  |  | 0.75 | 0.00 | 0.95 |  |  |  |  |

Figure A-5: Traffic Volume Data - CR 702 at New Road
DELAWARE VALLEY REGIONAL PLANNING COMMISSION
OFFICE OF TRAVEL M AM INTERVAL COUNTS
East-West Street
BERLIN AVE


DELAWARE VALLEY REGIONAL PLANNING COMMISSION
OFFICE OF TRAVEL M AM INTERVAL COUNTS
$\begin{array}{lll}\text { North-South Street } & \& & \text { East-West Street } \\ \text { NEW RD / STATION ENT } & & \text { BERLIN AVE }\end{array}$
CAMDEN
LINDENWO
$1 / 14 / 09$
WEDNESDAY
FAIR
5PM

Figure A-6: Traffic Volume Data - US 30 at New Road
$\infty$

North-South Street
WHITE HORSE PIKE
10/22/08
WEDNESDAY
FAIR
6AM

AM INTERVAL COUNTS - GYId GS\&OH GLIHM
 $\rightarrow 000 N O O H-N$ Hoctor
 INTERSECTION:
STREETS: East-West Street
JUGHANDLE / NEW ST

 COUNTY:
MUNICIPALITY:
CAMDEN
LINDENWOLD





Wd9

## CAMDEN LINDENWOLD

 -> 10/22/08 WEDNESDAY FAIR PM INTERVAL COUNTS

STARTING
TIME
$\begin{array}{ll}\text { 3:00 } & 3: 15 \\ 3 \cdot 15 & 3 \cdot 30\end{array}$
ले ले ले

Figure A-7: Traffic Volume Data - CR 673 at Central Avenue
DELAWARE VALLEY REGIONAL PLANNING COMMISSION
OFFICE OF TRAVEL MAM INTERVAL COUNTS
$\begin{array}{ll}\text { \& } & \text { East-West Street } \\ \text { CENTRAL AVE / UMDNJ ENTRANCE }\end{array}$





$$
\rightarrow N \text { HoOOONOHOHT } \infty \quad \text { กิ }
$$

$\infty$


10/22/08
WEDNESDAY
FAIR
7AM
7AM :SLAGYLS
:NOILOGS\&GLNI
:XLITV dIJINOW
:XLNOOJ

## DATE: DAY: WEATHER: FILE NUMBER

AM INTERVAL COUNTS
STARTING
TIME
6:00 $6: 15$
$?$
$\stackrel{0}{6}$
$\stackrel{\circ}{6}$



DELAWARE VALLEY REGIONAL PLANNING COMMISSION

## East-West Street CENTRAL AVE / UMDNJ ENTRANCE

8
CAMDEN
North-South Street
LAUREL RD
10/22/08
WEDNESDAY
FAIR
7PM

|  |  | LAUREL RD |  |  |  |  |  |  |  | CENTRAL AVE / UMDNJ ENTRANCE |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| STARTING |  | 1-NORTHBOUND |  |  |  | 2-SOUTHBOUND |  |  |  | 3-EASTBOUND |  |  |  |  | 4-WESTBOUND |  |  | N-S | E-WTOTAL | TOTAL |
| TIME |  | L | S | R | TOTAL | L | S | R | TOTAL | L | S | R | TOTAL | L | S | R | TOTAL | TOTAL |  |  |
| 3:00 3:15 |  | 2 | 139 | 7 | 148 | 18 | 134 | 3 | 155 | 1 | 1 | 2 | 4 | 8 | 0 | 15 | 23 | 303 | 27 | 330 |
| 3:15 3:30 |  | 5 | 124 | 8 | 137 | 10 | 103 | 7 | 120 | 2 | 1 | 0 | 3 | 3 | 2 | 39 | 44 | 257 | 47 | 304 |
| 3:30 3:45 |  | 2 | 172 | 7 | 181 | 13 | 157 | 3 | 173 | 2 | 0 | 1 | 3 | 6 | 2 | 31 | 39 | 354 | 42 | 396 |
| 3:45 4:00 |  | 2 | 193 | 10 | 205 | 15 | 171 | 4 | 190 | 2 | 0 | 1 | 3 | 1 | 0 | 28 | 29 | 395 | 32 | 427 |
| 4:00 4:15 |  | 0 | 145 | 9 | 154 | 20 | 114 | 2 | 136 | 1 | 0 | 0 | 1 | 12 | 1 | 36 | 49 | 290 | 50 | 340 |
| 4:15 4:30 |  | 2 | 172 | 6 | 180 | 14 | 140 | 0 | 154 | 2 | 0 | 0 | 2 | 10 | 3 | 22 | 35 | 334 | 37 | 371 |
| 4:30 4:45 |  | 1 | 175 | 7 | 183 | 17 | 158 | 2 | 177 | 0 | 0 | 1 | 1 | 14 | 2 | 31 | 47 | 360 | 48 | 408 |
| 4:45 5:00 |  | 2 | 167 | 5 | 174 | 19 | 146 | 0 | 165 | 0 | 0 | 0 | 0 | 8 | 3 | 35 | 46 | 339 | 46 | 385 |
| 5:00 5:15 |  | 2 | 162 | 12 | 176 | 16 | 138 | 1 | 155 | 1 | 1 | 0 | 2 | 15 | 1 | 56 | 72 | 331 | 74 | 405 |
| 5:15 5:30 |  | 1 | 144 | 3 | 148 | 8 | 143 | 1 | 152 | 0 | 1 | 1 | 2 | 12 | 2 | 48 | 62 | 300 | 64 | 364 |
| 5:30 5:45 |  | 4 | 149 | 6 | 159 | 6 | 144 | 1 | 151 | 0 | 0 | 1 | 1 | 5 | 2 | 19 | 26 | 310 | 27 | 337 |
| 5:45 6:00 |  | 2 | 162 | 5 | 169 | 8 | 124 | 0 | 132 | 0 | 0 | 0 | 0 | 5 | 0 | 16 | 21 | 301 | 21 | 322 |
| 6:00 6:15 |  | 0 | 152 | 3 | 155 | 6 | 161 | 0 | 167 | 1 | 0 | 2 | 3 | 2 | 2 | 14 | 18 | 322 | 21 | 343 |
| 6:15 6:30 |  | 2 | 170 | 5 | 177 | 10 | 159 | 1 | 170 | 0 | 0 | 1 | 1 | 6 | 2 | 19 | 27 | 347 | 28 | 375 |
| 6:30 6:45 |  | 1 | 148 | 5 | 154 | 9 | 133 | 2 | 144 | 1 | 1 | 0 | 2 | 7 | 1 | 15 | 23 | 298 | 25 | 323 |
| 6:45 7:00 |  | 1 | 155 | 4 | 160 | 7 | 117 | 1 | 125 | 0 | 0 | 0 | 0 | 4 | 2 | 15 | 21 | 285 | 21 | 306 |
|  | TOTALS | 29 | 2529 | 102 | 2660 | 196 | 2242 | 28 | 2466 | 13 | 5 | 10 | 28 | 118 | 25 | 439 | 582 | 5126 | 610 | 5736 |
|  | Peak Hour totals |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4:15-5:15 pm |  | 7 | 676 | 30 |  | 66 | 582 | 3 |  | 3 | 1 | 1 |  | 47 | 9 | 144 |  |  |  | 1569 |
| PHF |  | 0.88 | 0.97 | 0.63 |  | 0.87 | 0.92 | 0.38 |  | 0.38 | 0.25 | 0.25 |  | 0.78 | 0.75 | 0.64 |  |  |  |  |
| 4:30-5:30 pm |  | 6 | 648 | 27 |  | 60 | 585 | 4 |  | 1 | 2 | 2 |  | 49 | 8 | 170 |  |  |  | 1562 |
| PHF |  | 0.75 | 0.93 | 0.56 |  | 0.79 | 0.93 | 0.50 |  | 0.25 | 0.50 | 0.50 |  | 0.82 | 0.67 | 0.76 |  |  |  |  |

Figure A-8: Traffic Volume Data - CR 673 at Medical Center Drive
DELAWARE VALLEY REGIONAL PLANNING COMMISSION
OFFICE OF TRAVEL M AM INTERVAL COUNTS
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|  | DELAWARE VALLEY REGIONAL PLANNING COMMISSION OFFICE OF TRAVEL M AM INTERVAL COUNTS |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| COUNTY: <br> MUNICIPALITY: | CAMDEN STRATFORD |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| INTERSECTION: STREETS: | North-South Street LAUREL RD |  |  | \& |  | East-West Street <br> KENNEDY HOSPITAL ENTRANCE |  |  |  |  |  |  |  |  |  |  |  |  |  |
| DATE: <br> DAY: <br> WEATHER: | $\begin{aligned} & \text { 10/27/08 } \\ & \text { MONDAY } \\ & \text { FAIR } \end{aligned}$ |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| FILE NUMBER: | 8AM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| AM INTERVAL COUNTS |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  | LAUREL RD |  |  |  |  |  |  |  | KENNEDY HOSPITAL ENTRANCE |  |  |  |  |  |  |  | N-S | E-W | TOTAL |
| STARTING | 1-NORTHBOUND |  |  |  |  | 2-SOUTHBOUND |  |  |  | 3-EASTBOUND |  |  |  | 4-WESTBOUND |  |  |  |  |  |
| TIME | L | S | R | TOTAL | L | S | R | TOTAL | L | S | R | TOTAL | L | S | R | TOTAL | TOTAL | TOTAL |  |
| 6:00 6:15 | 0 | 87 | 9 | 96 | 11 | 23 | 0 | 34 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 3 | 130 | 3 | 133 |
| 6:15 6:30 | 0 | 130 | 13 | 143 | 15 | 25 | 0 | 40 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 183 | 0 | 183 |
| 6:30 6:45 | 0 | 144 | 16 | 160 | 31 | 44 | 0 | 75 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 2 | 235 | 2 | 237 |
| 6:45 7:00 | 0 | 174 | 45 | 219 | 54 | 35 | 0 | 89 | 0 | 0 | 0 | 0 | 1 | 0 | 5 | 6 | 308 | 6 | 314 |
| 7:00 7:15 | 0 | 168 | 18 | 186 | 34 | 48 | 0 | 82 | 0 | 0 | 0 | 0 | 1 | 0 | 4 | 5 | 268 | 5 | 273 |
| 7:15 7:30 | 0 | 172 | 20 | 192 | 21 | 76 | 0 | 97 | 0 | 0 | 0 | 0 | 0 | 0 | 7 | 7 | 289 | 7 | 296 |
| 7:30 7:45 | 0 | 191 | 14 | 205 | 30 | 111 | 0 | 141 | 0 | 0 | 0 | 0 | 6 | 0 | 15 | 21 | 346 | 21 | 367 |
| 7:45 8:00 | 0 | 157 | 31 | 188 | 47 | 101 | 0 | 148 | 0 | 0 | 0 | 0 | 8 | 0 | 13 | 21 | 336 | 21 | 357 |
| 8:00 8:15 | 0 | 179 | 31 | 210 | 50 | 97 | 0 | 147 | 0 | 0 | 0 | 0 | 3 | 0 | 4 | 7 | 357 | 7 | 364 |
| 8:15 8:30 | 0 | 200 | 23 | 223 | 24 | 105 | 0 | 129 | 0 | 0 | 0 | 0 | 5 | 0 | 7 | 12 | 352 | 12 | 364 |
| 8:30 8:45 | 0 | 190 | 11 | 201 | 38 | 99 | 0 | 137 | 0 | 0 | 0 | 0 | 1 | 0 | 7 | 8 | 338 | 8 | 346 |
| 8:45 9:00 | 0 | 134 | 5 | 139 | 20 | 72 | 0 | 92 | 0 | 0 | 0 | 0 | 1 | 0 | 6 | 7 | 231 | 7 | 238 |
| TOTALS | 0 | 1926 | 236 | 2162 | 375 | 836 | 0 | 1211 | 0 | 0 | 0 | 0 | 27 | 0 | 72 | 99 | 3373 | 99 | 3472 |
| Peak Hour totals |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7:30-8:30 am | 0 | 727 | 99 |  | 151 | 414 | 0 |  | 0 | 0 | 0 |  | 22 | 0 | 39 |  |  |  | 1452 |
| PHF | 0.00 | 0.91 | 0.80 |  | 0.76 | 0.93 | 0.00 |  |  |  |  |  | 0.69 | 0.00 | 0.65 |  |  |  |  |
| 7:45-8:45 am | 0 | 726 | 96 |  | 159 | 402 | 0 |  | 0 | 0 | 0 |  | 17 | 0 | 31 |  |  |  | 1431 |
| PHF | 0.00 | 0.91 | 0.77 |  | 0.80 | 0.96 | 0.00 |  |  |  |  |  | 0.53 | 0.00 | 0.60 |  |  |  |  |

Figure A-8: Traffic Volume Data - CR 673 at Medical Center Drive (Continued)
DELAWARE VALLEY REGIONAL PLANNING COMMISSION
OFFICE OF TRAVEL M AM INTERVAL COUNTS
East-West Street
KENNEDY HOSPITAL ENTRANCE

Figure A－9：Traffic Volume Data－CR 686 at CR 702－Berlin Road
DELAWARE VALLEY REGIONAL PLANNING COMMISSION
OFFICE OF TRAVEL M AM INTERVAL COUNI
\＆$\quad \begin{aligned} & \text { East－West Street } \\ & \text { GIBBSBORO RD }\end{aligned}$

CAMDEN
LINDENWOLD
North－South Street
BERLIN RD
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FAIR
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TOTALS
Peak Hour totals

7：00－8：00 am
PHF

System Peak
8：00－9：00 am
PHF
Figure A-9: Traffic Volume Data - CR 686 at CR 702-Berlin Road (Continued)

Figure A-10: Traffic Volume Data - CR 686 at CR 702-Egg Harbor Road
PLANNING COMMISSION
East-West Street
CARLTON ST / EGG HARBOR RD




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| 采号の |  | $\stackrel{\rightharpoonup}{\sim}$ |
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Table A-1: AM System Peak Hour Volumes
AM Peak Hour of System

| STARTING <br> TIME | INTERSECTION \#1 - White Horse | \#2-Laurel | \#2a-Laurel | \#2b - White Horse | \#3-Station | \#4-Berlin | \#5 - Berlin | \#5a-New | \#6 - White Horse | \#7-Laurel | \#8-Laurel | TOTALS | HOURLY TOTALS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 6:00 6:15 | 167 | 347 | 4 | 2 | 6 | 59 | 118 | 2 | 228 | 159 | 133 | 1225 |  |
| 6:15 6:30 | 226 | 454 | 4 | 7 | 11 | 114 | 159 | 4 | 325 | 173 | 183 | 1660 |  |
| 6:30 6:45 | 293 | 597 | 2 | 7 | 9 | 178 | 229 | 8 | 325 | 249 | 237 | 2134 |  |
| 6:45 7:00 | 372 | 649 | 4 | 8 | 12 | 247 | 118 | 10 | 355 | 266 | 314 | 2355 | 7374 |
| 7:00 7:15 | 326 | 751 | 7 | 6 | 13 | 119 | 206 | 18 | 229 | 287 | 273 | 2235 | 8384 |
| 7:15 7:30 | 457 | 817 | 14 | 10 | 24 | 156 | 157 | 15 | 280 | 319 | 296 | 2545 | 9269 |
| 7:30 7:45 | 540 | 923 | 21 | 10 | 31 | 278 | 210 | 24 | 375 | 358 | 367 | 3137 | 10272 |
| 7:45 8:00 | 562 | 942 | 18 | 7 | 25 | 236 | 226 | 29 | 472 | 366 | 357 | 3240 | 11157 |
| 8:00 8:15 | 590 | 833 | 30 | 14 | 44 | 348 | 317 | 18 | 436 | 345 | 364 | 3339 | 12261 |
| 8:15 8:30 | 519 | 871 | 39 | 11 | 50 | 255 | 152 | 27 | 426 | 380 | 364 | 3094 | 12810 |
| 8:30 8:45 | 565 | 828 | 49 | 15 | 64 | 308 | 221 | 10 | 428 | 398 | 346 | 3232 | 12905 |
| 8:45 9:00 | 514 | 779 | 29 | 9 | 38 | 294 | 277 | 13 | 327 | 350 | 238 | 2868 | 12533 |











INTERSECTION
 ©



STARTING
TIME
Figure A-14: Station Avenue Signal Warrant Volumes
DELAWARE VALLEY REGIONAL PLANNING COMMISSION OFFICE OF TRAVEL MAM INTERVAL COUNTS

| CI GSEAOH GLIHM <br>  |
| :---: |
|  |  |

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> CAMDEN
SOMERDALE
> North-South Street
> STATION AVE

1/13/09
1/13/09
TUESDAY
FAIR
3AM

- COUNTS

FILE NUMBER:

## DATE: DAY:

WEATHER:
STARTING
STARTING
TIME
6:00 6:15
$\begin{array}{ll}\text { 6:00 } & 6: 15 \\ 6: 15 & 6: 30\end{array}$

$00 \quad 7.15$
$\begin{array}{ll} & 7: 15 \\ 15 & 7: 30 \\ 7: 45 \\ 8: 00\end{array}$

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Figure A－14：Station Avenue Signal Warrant Volumes（Continued）

## DELAWARE VALLEY REGIONAL PLANNING COMMISSION <br> OFFICE OF TRAVEL M AM INTERVAL COUNTS

3PM

## COUNTY： MUNICIPALITY： <br> ：NOILOヨS\＆ヨLNI <br> STREETS：

North－South Street


WHITE HORSE RD

上 1000000000000 NNOOOOO


Figure A-15: Station Avenue Signal Warrant Summary


Figure A-15: Station Avenue Signal Warrant Summary (Continued)


APPENDIX B

## Alternative Recommendation Analysis

## Analysis of Possible Alternative Recommendations

As discussed in the Analysis and Recommended Improvements section, the LOS for existing conditions was compared against potential alternative improvements to determine the best recommendation for each intersection. The following tables show the average delay and corresponding LOS for each scenario at the intersections analyzed.
Table B-1: CR 673 (White Horse Road) at CR 702 (Berlin Road)

|  |  | Existing <br> Existing Timing and Geometry |  | Possible Alternative Recommendations |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Add SB Right-Turn Lane (250'); Assume Curb Lane Extended and PM Signal Added at Station Ave. | Restrict Turns at US 30; Assume Curb Lane Extended and PM Signal Added at Station Ave. |  |  |  | Add SB Right-Turn Lane (250'); Restrict Turns at US 30; Assume Curb Lane Extended and PM Signal Added at Station Ave. |  |  |  |
|  |  |  |  | Existing (120 Sec. CL ) |  | Existing (120 sec. CL) |  | Optimize Timing and Modify Phasing to <br> Leading Left (120 sec. CL) |  | Existing (120 sec. CL) |  | Optimize Timing and Modify Phasing to Leading Left (120 $\mathrm{sec} . \mathrm{CL})$ |  | Existing (120 sec. CL) |  | Optimize Timing and Modify Phasing to <br> Leading Left (120 sec. CL) |  |
|  |  | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS |
|  | CR 673 (NB) | 15 | B | 15 | B | 15 | B | 15 | B | 17 | B | 15 | B | 17 | B |
|  | CR 673 (SB) | 92 | F | 79 | E | 67 | E | 46 | D | 41 | D | 39 | D | 42 | D |
|  | CR 702 (EB) | 30 | C | 40 | D | 47 | D | 21 | C | 21 | C | 23 | C | 22 | C |
|  | CR 702 (WB) | 15 | B | 17 | B | 21 | C | 11 | B | 13 | B | 12 | B | 12 | B |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  | Overall Intersection | 37 | D | 34 | C | 33 | C | 24 | C | 23 | C | 22 | C | 23 | C |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  | Existing (120 Sec. CL) |  | Existing (120 sec. CL) |  | Optimize Timing and Modify Phasing to Leading Left (120 sec. CL) |  | Existing (120 sec. CL) |  | Optimize Timing and Modify Phasing to Leading Left (120 $\mathrm{sec} . \mathrm{CL})$ |  | Existing (120 sec. CL) |  | Optimize Timing and Modify Phasing to <br> Leading Left (120 sec. CL) |  |
|  |  | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS |
|  | CR 673 (NB) | 13 | B | 15 | B | 16 | B | 14 | B | 19 | B | 15 | B | 19 | B |
|  | CR 673 (SB) | 80 | E | 71 | E | 76 | E | 44 | D | 33 | C | 34 | C | 34 | C |
|  | CR 702 (EB) | 35 | D | 41 | D | 150 | F | 27 | C | 29 | C | 27 | C | 29 | C |
|  | CR 702 (WB) | 649 | F | 757 | F | 1660 | F | 27 | C | 133 | F | 26 | C | 154 | F |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  | Overall Intersection | 217 | F | 220 | F | 360 | F | 30 | C | 60 | E | 27 | C | 67 | E |
|  |  |  |  | Alt. 1 |  | Alt. 1a |  | Alt. 3 |  | Alt. 3a |  | Alt. 2 |  | Alt. 2a |  |

Table B-2: US 30 at CR 673 (White Horse Road)

|  |  | Existin |  | Possible Alternative Recommendations |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Existing Timing and Geometry |  | Add SB Right-Turn Lane (250'); Assume Curb Lane Extended and PM Signal Added at Station Ave. |  |  |  | Restrict Turns at US 30; Assume Curb Lane Extended and PM Signal Added at Station Ave. |  |  |  | Add SB Right-Turn Lane (250'); Restrict Turns at US 30; Assume Curb Lane Extended and PM Signal Added at Station Ave. |  |  |  |
|  |  | Existing (120 Sec.$\mathrm{CL})$ |  | Existing (120 sec. <br> CL ) |  | $\begin{array}{\|c} \hline \text { Optimize Timing } \\ \text { and Modify } \\ \text { Phasing to } \\ \text { Leading Left (120 } \\ \text { sec. CL) } \\ \hline \end{array}$ |  | Existing (120 sec. <br> CL ) |  | Optimize Timing and Modify Phasing to Leading Left (120 $\mathrm{sec} . \mathrm{CL}$ ) |  | Existing (120 sec. <br> CL ) |  | Optimize Timing and Modify Phasing to Leading Left (120 sec. CL) |  |
|  |  | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS |
|  | CR 673 (NB) | 33 | C | 32 | C | 34 | C | 40 | D | 41 | D | 39 | D | 41 | D |
|  | CR 673 (SB) | 34 | C | 32 | C | 39 | D | 37 | D | 52 | D | 30 | C | 52 | D |
|  | US 30 (EB) | 15 | B | 15 | B | 15 | B | 10 | B | 10 | B | 11 | B | 12 | B |
|  | US 30 (WB) | 19 | B | 19 | B | 19 | B | 13 | B | 13 | B | 13 | B | 13 | B |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  | Overall Intersection | 22 | C | 22 | C | 24 | C | 20 | C | 23 | C | 19 | B | 24 | C |
|  |  | Existing (120 $\mathrm{CL})$ |  | Existing (120 CL ) | 0 sec . | Optimize T <br> and Mod <br> Phasing <br> Leading Left $\mathrm{sec} . \mathrm{CL}$ | iming <br> dify <br> to <br> (120 <br> ) | Existing (120 CL ) |  | Optimize Ti <br> and Mod <br> Phasing <br> Leading Lef $\mathrm{sec} . \mathrm{CL}$ | ming ify <br> to | Existing (12 $\mathrm{CL})$ |  | Optimize T <br> and Mod <br> Phasing <br> Leading Left sec. CL | ming ify <br> to <br> (120 |
|  |  | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS |
|  | CR 673 (NB) | 43 | D | 44 | D | 60 | E | 41 | D | 44 | D | 41 | D | 43 | D |
|  | CR 673 (SB) | 55 | D | 54 | D | 84 | F | 39 | D | 66 | E | 29 | C | 70 | E |
|  | US 30 (EB) | 18 | B | 18 | B | 135 | F | 17 | B | 16 | B | 17 | B | 15 | B |
|  | US 30 (WB) | 14 | B | 14 | B | 14 | B | 13 | B | 13 | B | 13 | B | 11 | B |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  | Overall Intersection | 26 | C | 26 | C | 83 | F | 24 | C | 28 | C | 22 | C | 28 | C |

Table B-3: CR 673 (White Horse Road) at Station Avenue

Table B-4: US 30 at New Road

|  |  | Existin |  |  |  |  |  |  |  | Pos | ssibl | le Alterna | ative | Recomm | men | dations |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Existing Tim and Geom | ming netry | Remove Jug | ghandle | e, add EB Let SE corner ra | eft-Turn radius | Lane, and m | modify | Remove J | Jughan mo | dle, Add EM dify SE corne | Lefter radiu | Turn Lanes, us |  | Remove J | ghand | le, Add E/W and mod | Left-T <br> dify SE | urn Lanes, a corner radius | dd SB | Thru/Right L | ane, |
| $\begin{aligned} & \text { ̀ } \\ & \text { 우 } \end{aligned}$ |  | Existing (120 $\mathrm{CL})$ | 0 sec . | Existing (120 $\mathrm{CL})$ | $0 \mathrm{sec} .$ | Remove S Phasing Optimized S Plan (105 CL ) | Split g Signal sec. | Remove S Phasing (protected Signal Plan sec. CL) | $\begin{aligned} & \text { Split } \\ & \mathrm{g} \\ & \mathrm{~N} / \mathrm{S}) \\ & (120 \\ & )^{2} \end{aligned}$ | Existing (120 <br> $\mathrm{CL})$ | $0 \mathrm{sec} .$ | Add E/W L Phase Optim Signal Plan sec. CL | $\begin{aligned} & \text { Left } \\ & \text { mized } \\ & (120 \\ & -)^{2} \end{aligned}$ | Remove S Phasing, E/W \& N/S Signal Plan sec. CL | $\begin{aligned} & \hline \text { Split } \\ & \text { Add } \\ & \text { leads } \\ & (120 \\ & \hline \text { ) } \end{aligned}$ | Existing (120 CL) | $0 \text { sec. }$ | Optimized split) Signal (105 sec. | $\begin{aligned} & \text { (still } \\ & \text { I Plan } \\ & \mathrm{CL} \text { ) } \end{aligned}$ | $\begin{gathered} \text { Remove S } \\ \text { Phasing } \\ \text { (protected } \\ \text { Signal Plan } \\ \text { sec. } C L \text { ) } \end{gathered}$ | $\begin{aligned} & \hline \text { plit } \\ & y \\ & \mathrm{~N} / \mathrm{S}) \\ & (120 \end{aligned}$ | All Protected Phases Optimized Plan (120 CL) | deft <br> Signal sec. |
| ¢ |  | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS |
| d | New St (NB) | 25 | C | 50 | D | 46 | D | 48 | D | 55 | D | 58 | E | 53 | D | 51 | D | 47 | D | 53 | D | 54 | D |
| Q | New St (SB) | 43 | D | 8 | A | 7 | A | 6 | A | 8 | A | 8 | A | 7 | A | 8 | A | 7 | A | 6 | A | 7 | A |
| $\sum$ | US 30 (EB) | 3 | A | 4 | A | 3 | A | 4 | A | 4 | A | 3 | A | 3 | A | 4 | A | 6 | A | 5 | A | 3 | A |
|  | US 30 (WB) | 8 | A | 10 | B | 6 | A | 9 | A | 9 | A | 14 | B | 11 | B | 10 | B | 10 | B | 9 | A | 12 | A |
|  | Overall Intersection | 10 | A | 8 | A | 6 | A | 7 | A | 8 | A | 9 | A | 8 | A | 8 | A | 9 | A | 8 | A | 8 | A |
| $\begin{aligned} & \text { ̀ } \\ & \text { 오 } \end{aligned}$ |  | Existing (120 $\mathrm{CL})$ | 0 sec . | Existing (120 $C L)$ | $0 \mathrm{sec} .$ | Remove S Phasing Optimized S Plan (105 CL ) | $\begin{aligned} & \hline \text { Split } \\ & \text { ig } \\ & \text { Signal } \\ & \text { sec. } \end{aligned}$ | Remove S Phasing (protected Signal Plan $\mathrm{sec} . \mathrm{CL}$ ) |  | Existing (120 <br> CL) | $0 \mathrm{sec} .$ | Add E/W L Phase Optim Signal Plan sec. CL | Left mized (120 ) | Remove S Phasing, E/W \& N/S Signal Plan sec. CL | Split Add leads (120 | Existing (120 $C L)$ | sec . | Optimized split) Signal (105 sec. | $\begin{aligned} & \text { (still } \\ & \text { I Plan } \\ & \mathrm{CL} \text { ) } \end{aligned}$ | Remove S Phasing (protected Signal Plan sec. $C L$ ) |  | All Protected Phases Optimized Plan (120 CL) | deft <br> Signal sec. |
| 둘 |  | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS |
| 0 | New St (NB) | 39 | D | 51 | D | 27 | C | 47 | D | 50 | D | 91 | F | 55 | D | 50 | D | 57 | E | 46 | D | 56 | E |
| ロ | New St (SB) | 120 | F | 129 | F | 61 | E | 41 | D | 122 | F | 115 | F | 47 | D | 117 | F | 220 | F | 41 | D | 59 |  |
| $\Sigma$ | US 30 (EB) | 16 | B | 13 | B | 9.0 | A | 11 | B | 12 | B | 9 | A | 10 | A | 11 | B | 8 | A | 11 | B | 10 | A |
|  | US 30 (WB) | 15 | B | 16 | B | 13 | B | 16 | B | 15 | B | 19 | B | 18 | B | 16 | B | 12 | B | 16 | B | 20 | B |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  | Overall Intersection | 45 | D | 40 | D | 21 | C | 20 | B | 37 | D | 38 | C | 20 | C | 37 | D | 51 | D | 20 | C | 25 | C |
|  |  | Existing (120 $\mathrm{CL})$ | sec . |  |  |  |  | Remove S Phasing (protected N Signal Plan sec. CL) | $\begin{aligned} & \text { Split } \\ & g \\ & \mathrm{~N} / \mathrm{S}) \\ & (120 \\ & \left.\mathrm{g}^{2}\right) \end{aligned}$ |  |  |  |  | Remove S Phasing, E/W \& N/S Signal Plan sec. CL | $\begin{aligned} & \text { Split } \\ & \text { Add } \\ & \text { leads } \\ & (120 \end{aligned}$ |  |  |  |  | $\begin{gathered} \text { Remove } \\ \text { Phasing } \\ \text { (protected } \\ \text { Signal Plan } \\ \text { sec. } C L \text { ) } \end{gathered}$ | plit <br> N/S) <br> (120 |  |  |
| ¢ |  | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS |
| E | New St (NB) | 67 | E |  |  |  |  | 91 | F |  |  |  |  | 122 | F |  |  |  |  | 72 | E |  |  |
| 2 | New St (SB) | 132 | F |  |  |  |  | 48 | D |  |  |  |  | 105 | F |  |  |  |  | 82 | F |  |  |
| $\frac{0}{0}$ | US 30 (EB) | 19 | B |  |  |  |  | 36 | D |  |  |  |  | 12 | B |  |  |  |  | 15 | B |  |  |
| 入 | US 30 (WB) | 30 | C |  |  |  |  | 54 | D |  |  |  |  | 21 | C |  |  |  |  | 24 | C |  |  |
| 0 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  | Overall Intersection | 57 | E |  |  |  |  | 48 | D |  |  |  |  | 41 | D |  |  |  |  | 36 | D |  |  |
| $\begin{aligned} & \sum \\ & \mathbf{N} \\ & N \\ & \# \\ & \pm \end{aligned}$ |  | Existing (120 <br> CL) | sec. |  |  |  |  | Remove S Phasing (protected N Signal Plan sec. CL) | $\begin{aligned} & \text { Split } \\ & g \\ & \mathrm{~N} / \mathrm{S}) \\ & (120 \\ & \hline \end{aligned}$ |  |  |  |  | Remove S Phasing, E/W \& N/S Signal Plan sec. CL | $\begin{aligned} & \text { Split } \\ & \text { Add } \\ & \text { leads } \\ & (120 \\ & \hline \end{aligned}$ |  |  |  |  | $\begin{array}{\|c} \hline \text { Remove } \mathrm{S} \\ \text { Phasing } \\ \text { (protected } \\ \text { Signal Plan } \\ \text { sec. } \mathrm{CL} \text { ) } \\ \hline \end{array}$ | $\begin{aligned} & \hline \text { split } \\ & g \\ & \mathrm{~N} / \mathrm{S}) \\ & (120 \end{aligned}$ |  |  |
| ¢ |  | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS | Delay (sec) | LOS |
| E | New St (NB) | 67 | E |  |  |  |  | 83 | F |  |  |  |  | 115 | F |  |  |  |  | 70 | E |  |  |
| 응 | New St (SB) | 107 | F |  |  |  |  | 43 | D |  |  |  |  | 49 | D |  |  |  |  | 52 | D |  |  |
| O | US 30 (EB) | 18 | B |  |  |  |  | 17 | B |  |  |  |  | 12 | B |  |  |  |  | 14 | B |  |  |
| Ј | US 30 (WB) | 24 | C |  |  |  |  | 28 | C |  |  |  |  | 20 | B |  |  |  |  | 27 | C |  |  |
| 0 |  | 50 | D |  |  |  |  | 31 | C |  |  |  |  | 29 | C |  |  |  |  |  |  |  |  |
|  | Overall intersection |  |  |  |  |  |  |  |  |  |  |  |  | 29 |  |  |  |  |  | 29 | c |  |  |

Table B-5: US 30 at New Road (continued)

Table B-6: CR 686 (Gibbsboro Road) at CR 702 (BerlinRoad/Egg Harbor Road)


APPENDIX C

## Potential Redevelopment Areas

## Analysis of Potential Redevelopment Areas

There are several potential redevelopment areas located within the study area. These areas are illustrated on Figure C-1.

## Development between Berlin Road and White Horse Pike

The Borough of Stratford indicated that there is potential for redevelopment in the area between Berlin Road and White Horse Pike along Hunt and Coolidge Avenues. While the specifics of this redevelopment are unknown, it was noted that the area will likely continue as a residential use and therefore should be connected with the Lindenwold Station. Elements such as sidewalks with curb ramps and crosswalks will provide needed connectivity to Berlin Road and the station.

## Development scenarios for the former Bradlee's site

The area at the south end of New Road, the former site of Bradlee's discount department store, has also been designated as a potential redevelopment area by the Borough of Stratford. In order to account for the development potential of this site in this study a range of expected trips was calculated. This range of trips generated was used to determine the impact of development on the intersection of U.S. 30 and New Road and how the recommendations made earlier for the intersection. Table C-1 shows the breakdown of trip ends for the two assumed scenarios used in the analysis.

The low value of the range assumed that the site would remain developed as it is now, with no additional construction. This scenario assumes that the vacant Bradlee's building, which is approximately 125,000 square feet, would be reused as a retail store. If this building were to be reused as a Discount Club (ITE Land Use Code \# 861 ${ }^{1}$ ), it would be reasonable to see 600 trips generated during the AM and PM peak hours combined.

The high value of the range assumed that the site would be redevoped with a mixed-use consisting of office, retail, and residential uses. This scenario assumes that the parcel is redeveloped as office space, a supermarket, and a high turnover restaurant; all with second floor residential apartments (ITE Codes \# 710, 850, 932, 220). Additionally, it was assumed that the restaurant would not be open during the AM peak hour. The mixed-use scenario would reasonably produce 1,219 trips in the combined AM and PM peak hours each day.

[^2]Table C-1: Trips Generated by Development Scenarios

| ITE Land Use | Assumed <br> Independent <br> Variable | Total Trips | Trips |  |
| :---: | :---: | :---: | :---: | :---: |
| Scenario \#1 |  |  |  |  |
| \#861 Discount Club | 125,000 Sq.Ft. | $70-\mathrm{AM}$ | 35 | 35 |
| Exiting |  |  |  |  |

Source: DVRPC 2009

C-2


| Publication Title: | Lindenwold Station Transit Hub Study |
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Key Words: transportation, transit service, PATCO, High Speed Line, NJ Transit, Lindenwold Station, intersection improvements, solutions, pedestrian access, level of service, bicycle facilities, connectivity, intersection analysis, crash analysis

| Abstract: | Lindenwold Station is the junction of two rail lines; and the Port Authority Transit Corporation (PATCO) terminates at Lindenwold. The Lindenwold Station boards for PATCO service in New Jersey. NJ Tra Philadelphia and Atlantic City has been growing s Recommendations are made by the study team to at the station and to support Lindenwold as a hub include improving pedestrian and bicycle access to integrated multimodal transportation network and bottlenecks and complex intersections along CR 67 |
| :---: | :---: |
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[^0]:    $1^{1}$ Increasing Intermodal Access to Transit, Phase I।

[^1]:    DELAWARE VALLEY REGIONAL PLANNING COMMISSION
    DELAWARE VALLEY REGIONAL PLANNING COMMISSION
    OFFICE OF TRAVEL M AM INTERVAL COUNTS

[^2]:    ${ }^{1}$ ITE Trip Generation, $7^{\text {th }}$ Edition.

